



December 4, 2023

Stockbridge Planning Board
50 Main Street
Stockbridge, MA 01262

RE: Definitive Subdivision – 35-37
Interlaken Rd.

Dear Planning Board Members;

Enclosed for your review, please find four (4) sets of Definitive Subdivision Plans for the above-named project. Also enclosed are the following documents:

1. Four (4) Copies of the Form "C" Application.
2. Copy of a Letter to the Town Clerk.
3. ¹Two (2) copies of the Stormwater Management Study dated 11/22/23.
4. Copy of original property survey plan (by Foresight Land Services, dated July 24, 2012).
5. Definitive Subdivision Plans (Dated Dec. 4, 2023).

We have inquired about a filing *fee* but have not been able to determine an amount. Once an amount is determined, we will arrange payment forthwith.

Per Stockbridge Subdivision Regulations, we have not prepared an Impact Statement because the project is less than 10 lots in size.

We look forward to presenting the project at the next available public hearing. If you have any questions, feel free to contact us.

Sincerely,

S-K DESIGN GROUP, INC.

Robert G. Fournier
[Project Manager]

Enclosures

CC: Stockbridge Conservation Commission (2 copies)
35-37 Interlaken Road Realty Trust
Atty. Jonathan M. Silverstein
File

¹ Additional copies available upon request

ATTACHMENT 1

Form C

FORM C
APPLICATION FOR APPROVAL OF DEFINITIVE PLAN

File one completed form with the Planning Board and one copy with the Town Clerk.

(Where alternative paragraphs are provided, applicant is to select and complete the paragraph pertinent to his case.)

TO THE PLANNING BOARD OF THE TOWN OF STOCKBRIDGE:

1. The undersigned applicant, being the owner of all land included within a proposed subdivision shown on the accompanying plan, entitled:

Definitive Subdivision, Prepared for 35-37 Interlaken Road Realty Trust

and dated _____, 20 23, and prepared by SK Design Group, Inc.

Massachusetts Registered (Engineer) (Surveyor), registration no. 39863
submits such plan as a definitive plan of the proposed subdivision and makes application to the Board for final approval thereof.

2. The land within the proposed subdivision is subject to the following easements and restrictions.

Book 544, Pg. 582; Book 532, Pg. 582;

Book 537, Pg. 377; Book 476, Pg. 524

3. There are appurtenant to the land within the proposed subdivision the following easements and restrictions over the land of others:

4. A preliminary plan of the proposed subdivision was discussed by the Board on June 6, 20 23.
The modifications recommended at this meeting have been incorporated in the accompanying plan.

5. The applicant agrees, if the definitive plan is approved, to construct and install all improvements within the proposed subdivision required by the rules and regulations of the Stockbridge
Planning Board as in force on the date of this application, and as modified and supplemented by the work specifications and other requirements of the Department of Public Works and the Health Dept.

6. The applicant further agrees to complete all said required improvements within two years from the date of approval of the definitive plan by the Board, unless the Board approves a different period of time.

7. The applicant further agrees, if this application is approved, to file with the Board within (20) days of such approval, a bond in form satisfactory to the Board and conditioned on the completion of all required improvements in the time and manner prescribed, in a penal sum sufficient, in the opinion of the Board, to cover the cost of such work, and executed by the applicant as principal and a surety company authorized to do business in the Commonwealth and satisfactory money or negotiable securities, satisfactory to the Board, in an amount equal to the penal sum of the bond.

OR

(As an alternative to the above agreement) The applicant requests the Board to approve the definitive plan on condition that no lot in the subdivision shall be sold and no building shall be erected or placed on any lot until the required improvements specified are constructed and installed so as to serve the lots adequately.

8. The applicant further agrees, if this application is approved, to cause the definitive plan of the subdivision to be recorded in the Berkshire Registry of Deeds or in the Berkshire Land Registry District within (6) months of such approval, and agrees not to sell, or to offer to sell, any of the lots within the subdivision.
9. The owner's title to the land is derived under deed from Lee Bank,
dated May 11, 2009, and recorded in
Berkshire District Registry of Deeds, book 04293 page 338
or under Land Court Certificate of Title no. _____
registered in Berkshire Land Registry District, book _____ page _____

Applicant 35-37 Interlaken Road Realty Trust

Address 776 Boylston St., Unit E9A, Boston, MA 02199

Accepted this _____ day of _____, 20____, as duly submitted under the rules and regulations of the Stockbridge Planning Board

Stockbridge Planning Board

BY _____

Fee of \$ _____ received _____, 20____

By _____,

TITLE

ATTACHMENT 2

Letter to Town Clerk



December 4, 2023

Stockbridge Town Clerk
50 Main Street
Stockbridge, MA 01262

RE: Definitive Subdivision – 35-37
Interlaken Rd.

Dear Clerk;

This letter is official notification of the **submission of a Definitive Subdivision Plan** to the Town of Stockbridge Planning Board. Said plan is entitled, "Building Lot Plan, of Bonnie Brier Lane, Surveyed for 35-37 Interlaken Road Realty Trust, Located at 35-37 Interlaken Rd., Stockbridge, Massachusetts, Scale: 1" = 40', December 4, 2023, S-K Design Group, Inc., Civil Engineers-Consultants-Surveyors, 2 Federico Drive, Pittsfield, Massachusetts 01201".

The plans are submitted in accordance with Section 81-T of Chapter 41 of the General Laws of Massachusetts.

Also attached, please find the Form "C" Application for Approval of the Definitive Plans with pertinent information thereon.

If you should have any questions or require additional information, please do not hesitate to contact our office.

Sincerely,

S-K DESIGN GROUP, INC.

Robert G. Fournier
[Project Manager]

Enclosures

CC: 35-37 Interlaken Road Realty Trust
Atty. Jonathan M. Silverstein
File

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ATTACHMENT 3

Stormwater Management Study

STORMWATER MANAGEMENT REPORT

Bonnie Brier Lane

Stockbridge, MA

Applicant:

35-37 Interlaken Road Realty Trust

Prepared by:

SK DESIGN GROUP INC

November 22, 2023

SUMMARY:

The project is a single-family subdivision of 3 lots on a proposed cul-de-sac off Interlaken Road, between Meadow Road and Tree Farm Road. The Stormwater Management System begins with deep-sump catch basins that collect runoff, to be piped to a treatment manhole. There, flows are treated with all flows up to the volume required for groundwater recharge and water quality treatment going to an infiltration basin. The restricted discharge from the infiltration basin leads to a level spreader. The stormwater management system complies with DEP standards in preventing increases in peak runoff, providing groundwater recharge, removing pollutants, controlling erosion and sediment during construction, and providing for long-term management. There will be no increase in peak rate of runoff to any of the abutting properties to the project.

APPLICABILITY

The Stormwater management standards are applied to this project generically based upon the design standards set forth under the Subdivision regulations. Reviewing the Massachusetts Stormwater Management Standards, we find that they do not apply to small projects such as this.

Applicability

Except as expressly provided herein, stormwater runoff from all industrial, commercial, institutional, office, residential and transportation projects including site preparation, construction and redevelopment, and all point source stormwater discharges from said projects shall be managed according to the Stormwater Management Standards. However, -

The Stormwater Management Standards shall not apply to:

- (1) A single-family house;*
- (2) Housing development and redevelopment projects comprised of detached single-family dwellings on four or fewer lots provided that there are no stormwater discharges that may potentially affect a critical area;*

In this case, application of the stormwater standards results in a review of said standards and a determination that they do not apply. Notwithstanding the inapplicability of the standards, the proposed subdivision meets the standards to the maximum practical extent.

EXISTING CONDITIONS:

Land Use and vegetative cover: The property has been developed since 1890 according to the local real estate records. The primary uses include residential and educational. The proposed development is limited to the portion of the lot between Interlaken Road and a protected wetland system that bisects the property. The property includes assessor lots #31 and #32. The picture below depicts the development area and cover, including buildings, lawn, and trees.



Soils: The soil on the site have been mapped by the USDA. We have adjusted the mapped boundaries based on a detailed topographic mapping. Adjusted soil boundaries and map codes are shown on the accompanying watershed maps. The soil series present are silty loam and gravel. On-site soils in the development area are rated as Hydrologic Soil Group (HSG) "C", Silty loam soils typically yield somewhat higher runoff rates.

Hydrologic Setting: Drainage from the proposed subdivision and the two proposed houses will flow easterly towards Interlaken Road. While other portions of the property drain to the south and west, no work is proposed in those areas.

Low Impact Development: The use of "Low Impact Development" practices is assumed in the project development. The individual house designs are undetermined, yet the site is conducive to the installation of rain gardens. Favorable soil and groundwater depth in the in the building envelopes is

expected to encourage infiltration. While the new lots exceed 10 acres in area, the building envelopes are restricted to two acres for purposes of this analysis. This will disconnect runoff and allow dispersion over undeveloped lawns as currently existing and avoids point source discharges related to the construction of houses. This approach avoids the need for structural BMPs.

Stormwater Management System Summary: Runoff from the subdivision road is captured in deep sump catch basins, routed through a stormceptor manhole and directed to the on-site infiltration basin. The basin will be constructed on private property and maintained by a Homeowner's Association.

DESIGN METHODOLOGY:

For watershed modeling and design of the detention/infiltration basin, Natural Resource Conservation Service methods, based on TR-55 and TR-20, were followed using the "HydroCad" computer program (ver. 10.0 build 12), an adaptation of SCS TR-20 and TR-55 methods. The drainage basin characteristics of Curve Number (CN) and time of concentration (Tc) are described and calculated based on NRCS methodology.

Summaries are provided for the 2-year, and 10-year storms. Details of parameters used for the model are provided in the section on the 100-year storm. All input parameters for the runoff estimate from the contributing areas are the same except for the rainfall.

COMPLIANCE WITH DEP STANDARDS:

The project design generally complies with MA DEP stormwater management standards, as summarized below.

Standard 1. No new untreated discharge or erosion: The new point source discharge from the project will receive over 80% TSS removal for water quality treatment in compliance with DEP guidelines. Erosion controls will be implemented during construction to prevent sediment discharge from the site (see site plans). The proposed flow rate from the detention basin to the roadside swale is lower than under current conditions, and the discharge will be dispersed by a stone spreader, eliminating erosion hazard. Discharge velocity from the 12" outlet, is within the control capacity of a good sod cover. The

stone stilling basin and spreader will further reduce the discharge velocities to well below any potentially erosive rates.

Standard 2. Post-development peak discharge rates do not exceed pre-development: Proposed peak flows from the site are reduced compared to existing flows as summarized below:

Discharge to the Design Point under Existing Conditions:

Event	Inflow (cfs)	Primary (cfs)
2 YR	9.88	9.88
10 YR	24.09	24.09
25 YR	36.07	36.07
100 YR	61.73	61.73

Discharge to the Design Point under Proposed Conditions:

Event	Inflow (cfs)	Primary (cfs)
2 YR	9.32	9.32
10 YR	21.63	21.63
25 YR	32.19	32.19
100 YR	55.09	55.09

In each case the post development design flows to the design point are less than the existing flows. The details of the stormwater analysis are found in the summaries from the HydroCAD calculations attached hereto.

Standard 3. Annual recharge of groundwater should approximate current conditions: Runoff from the subdivision road is captured and directed to the infiltration basin after treatment. The runoff from future house development is disconnected in 20% of the lot area to promote natural attenuation and recharge consistent with LID practices. No point source discharges are proposed from the building lots.

$$Rv = F \times \text{impervious area}$$

Rv = Required Recharge Volume, expressed in Ft³, cubic yards, or acre-feet

F = Target Depth Factor associated with each Hydrologic Soil Group

Impervious Area = pavement and rooftop area on site

$$Rv = [(0.25\text{-in}/12)(0.56 \text{ acres})]$$

$$Rv = 0.01157 \text{ acre-feet}$$

$$Rv = 0.01157 \text{ acre-feet} \times 43560 \text{ square feet/acre-foot} = 504 \text{ cubic feet Total}$$

Capture area adjustment: determining if enough runoff is directed to the recharge practice

In this case, the site design directs only a portion of the site's impervious area to the BMPs. As a result, the infiltration BMPs may not be able to capture sufficient rainfall on an average annual basis to meet the *Required Recharge Volume*. This design increases the storage capacity of the infiltration BMPs so that they may capture more of the runoff from the impervious surfaces located within the contributing drainage area. The procedure for this is as follows:

- 1) $Rv = F \times \text{impervious area}$
- 2) $Rv = [(0.25 \text{ inches}/12 \text{ inches/foot})(1.91 \text{ acre})(43,560 \text{ sq. ft./acre})]$
 $Rv = 1,735 \text{ cubic feet}$
- 3) $\text{Site area draining to recharge facilities} = .56 \text{ acres}$
- 4) $\text{Ratio of total site area to site area draining to recharge facilities} = 1.91 \text{ acre}/0.56 \text{ acre} = 3.4$
- 5) $\text{Adjusted minimum required recharge volume} = [(3.4)(1735 \text{ cubic feet})] = 5917 \text{ cu. ft.}$

The detention basin provides infiltration but does not meet the separation to groundwater requirement of 2 feet seasonally. The basin provides 10,118 cf of storage below the primary outlet invert. The project requests relief from this standard under the redevelopment provisions the inapplicability of the standards for this development based upon its small size.

$$Time_{drawdown} = \frac{1735 \text{ cubic feet}}{(0.3 \text{ inches/hour})(1\text{ft}/12 \text{ inches})(4188 \text{ square feet})}$$

$$Time_{drawdown} = 17 \text{ hours} < 72 \text{ Hours OK}$$

The standards require the drawdown occur in 72 hours the BMPs for recharge are in the hydrologic soil group 'C' soils which provide adequate infiltration rates.

Standard 4. 80% Total Suspended Solids (TSS) removal: All point source discharges from impervious Subdivision Road surfaces are treated to remove over 80% of Total Suspended Solids (TSS). All

impervious areas tributary to the discharge are routed through the stormwater management system.

Location: Bonnie Brier Lane

A BMP ¹	B TSS Removal Rate ¹	C Starting TSS Load*	D Amount Removed (B*C)	E Remaining Load (C-D)
Deep Sump Catch Basin	0.25	1.00	0.25	0.75
Stormceptor	0.80	0.75	0.60	0.15
Detention Basin	0.50	0.15	0.075	0.075

Total TSS Removal = 92.5%

Project: Stockbridge Sub

Prepared By: Jim Scalise

Date: Nov. 2023

*Equals remaining load from previous BMP (E)
which enters the BMP

Separate Form Needs to be Completed for Each Outlet or BMP Train

Standard 5. Higher potential pollutant loadings prohibit certain practices: Not Applicable

Standard 6. Discharges to critical areas treat 1" of runoff and prohibit certain practices: Not Applicable

Standard 7. Redevelopment sites must meet standards to maximum extent practicable and improve Existing conditions: This is applicable although the project is not jurisdictional under the standard and a mix of new and re-development. This standard is not specifically applied yet is an acceptable justification for not meeting the full standards.

Standard 8. Construction related Impacts including Erosion and Sediment Controls must be implemented. A detailed erosion and sediment control procedure is spelled out for the project on the plan set. This includes perimeter sediment barriers, rough grading of lots to provide depressions for

sediment capture, and interim seeding of rough graded lots for stabilization until final construction is done on the lots. Until construction is completed and all tributary areas are stabilized, frequent inspection and maintenance of the erosion controls is required. Construction period maintenance will be the responsibility of the site work contractor. A construction period "Stormwater Pollution Prevention Plan" (SWPPP) will be prepared and submitted by the developer prior to any site work taking place.

Standard 9. Long Term Operation and Maintenance Plan required: See Attachment.

After construction, a minimum of annual inspection and maintenance of all system components is recommended. Maintenance of stormwater facilities outside of the public way will be the responsibility of the homeowners' association. The only vegetation management required is annual mowing of the detention and infiltration basins and adjacent slopes to prevent development of large woody vegetation and maintain a good sod cover. The infiltration basin is designed so that it can be maintained as lawn by the Association. The assumed infiltration rate makes allowance for compaction by regular mowing and foot traffic. The stormwater management facilities should be inspected monthly during the first year of operation or until it is clear they are functioning properly. After that, inspections should generally be twice a year, with maintenance as required. With proper construction and maintenance, clogging of the infiltration surface of the infiltration basin is not expected to be a problem. However, if ponding persists in the basin for over 72 hours, the basin should be de-watered (pump to roadside swale) and roto-tilled to restore the infiltration capacity, and re-seeded.

Standard 10. Illicit discharges prohibited: No illicit discharges are known to exist on the site, and the proposed project has no potential sources of illicit discharge. Sanitary sewerage will be collected by the gravity sewer system as shown on the plans.

Detailed Stormceptor Sizing Report – Sub Area 1: parking area

Project Information & Location			
Project Name	Bousquet	Project Number	200201
City	Pittsfield	State/ Province	Massachusetts
Country	United States of America	Date	8/3/2022
Designer Information		EOR Information (optional)	
Name	James Scalise	Name	
Company	SK Design Group, Inc	Company	
Phone #	413-443-3537	Phone #	
Email	jscalise@sk-designgroup.com	Email	

Stormwater Treatment Recommendation

The recommended Stormceptor Model(s) which achieve or exceed the user defined water quality objective for each site within the project are listed in the below Sizing Summary table.

Site Name	Sub Area 1: parking area
Recommended Stormceptor Model	STC 450i
Target TSS Removal (%)	80.0
TSS Removal (%) Provided	83
PSD	Fine Distribution
Rainfall Station	PITTSFIELD WB AIRPORT

The recommended Stormceptor model achieves the water quality objectives based on the selected inputs, historical rainfall records and selected particle size distribution.

Stormceptor Sizing Summary	
Stormceptor Model	% TSS Removal Provided
STC 450i	83
STC 900	89
STC 1200	89
STC 1800	90
STC 2400	92
STC 3600	93
STC 4800	94
STC 6000	94
STC 7200	96
STC 11000	97
STC 13000	97
STC 16000	98

Stormceptor

The Stormceptor oil and sediment separator is sized to treat stormwater runoff by removing pollutants through gravity separation and flotation. Stormceptor's patented design generates positive TSS removal for each rainfall event, including large storms. Significant levels of pollutants such as heavy metals, free oils and nutrients are prevented from entering natural water resources and the re-suspension of previously captured sediment (scour) does not occur.

Stormceptor provides a high level of TSS removal for small frequent storm events that represent the majority of annual rainfall volume and pollutant load. Positive treatment continues for large infrequent events, however, such events have little impact on the average annual TSS removal as they represent a small percentage of the total runoff volume and pollutant load.

Design Methodology

Stormceptor is sized using PCSWMM for Stormceptor, a continuous simulation model based on US EPA SWMM. The program calculates hydrology using local historical rainfall data and specified site parameters. With US EPA SWMM's precision, every Stormceptor unit is designed to achieve a defined water quality objective. The TSS removal data presented follows US EPA guidelines to reduce the average annual TSS load. The Stormceptor's unit process for TSS removal is settling. The settling model calculates TSS removal by analyzing:

- Site parameters
- Continuous historical rainfall data, including duration, distribution, peaks & inter-event dry periods
- Particle size distribution, and associated settling velocities (Stokes Law, corrected for drag)
- TSS load
- Detention time of the system

Hydrology Analysis

PCSWMM for Stormceptor calculates annual hydrology with the US EPA SWMM and local continuous historical rainfall data. Performance calculations of Stormceptor are based on the average annual removal of TSS for the selected site parameters. The Stormceptor is engineered to capture sediment particles by treating the required average annual runoff volume, ensuring positive removal efficiency is maintained during each rainfall event, and preventing negative removal efficiency (scour). Smaller recurring storms account for the majority of rainfall events and average annual runoff volume, as observed in the historical rainfall data analyses presented in this section.

Rainfall Station

State/Province	Massachusetts	Total Number of Rainfall Events	3838
Rainfall Station Name	PITTSFIELD WB AIRPORT	Total Rainfall (in)	847.0
Station ID #	6414	Average Annual Rainfall (in)	36.8
Coordinates	42°26'0"N, 73°17'0"W	Total Evaporation (in)	102.9
Elevation (ft)		Total Infiltration (in)	16.6
Years of Rainfall Data	23	Total Rainfall that is Runoff (in)	727.5

Notes

- Stormceptor performance estimates are based on simulations using PCSWMM for Stormceptor, which uses the EPA Rainfall and Runoff modules.
- Design estimates listed are only representative of specific project requirements based on total suspended solids (TSS) removal defined by the selected PSD, and based on stable site conditions only, after construction is completed.
- For submerged applications or sites specific to spill control, please contact your local Stormceptor representative for further design assistance.

Drainage Area	
Total Area (acres)	0.7
Imperviousness %	98.0

Water Quality Objective	
TSS Removal (%)	80.0
Runoff Volume Capture (%)	
Oil Spill Capture Volume (Gal)	
Peak Conveyed Flow Rate (CFS)	5.40
Water Quality Flow Rate (CFS)	0.91

Up Stream Storage	
Storage (ac-ft)	Discharge (cfs)
0.000	0.000

Up Stream Flow Diversion	
Max. Flow to Stormceptor (cfs)	

Design Details	
Stormceptor Inlet Invert Elev (ft)	1114.40
Stormceptor Outlet Invert Elev (ft)	1114.40
Stormceptor Rim Elev (ft)	1119.40
Normal Water Level Elevation (ft)	1112.00
Pipe Diameter (In)	12
Pipe Material	HDPE - plastic
Multiple Inlets (Y/N)	No
Grate Inlet (Y/N)	No

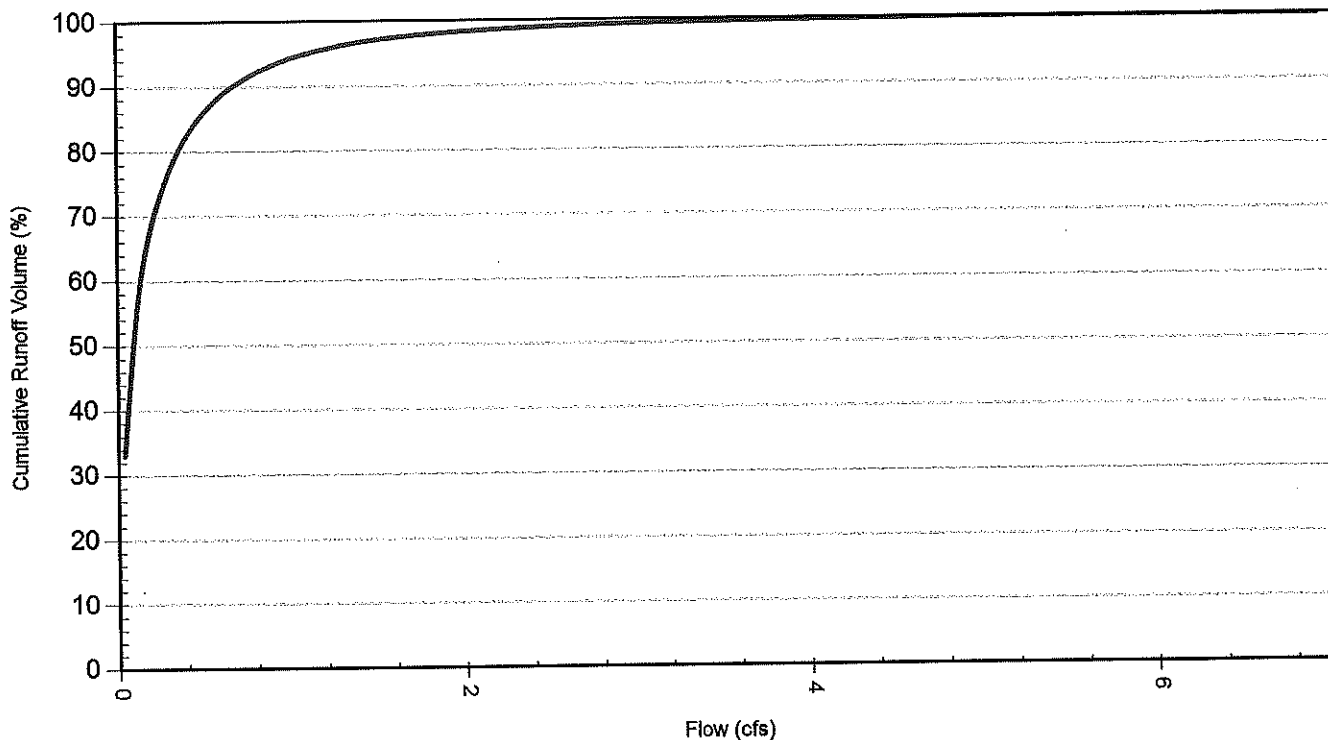
Particle Size Distribution (PSD)		
Removing the smallest fraction of particulates from runoff ensures the majority of pollutants, such as metals, hydrocarbons and nutrients are captured. The table below identifies the Particle Size Distribution (PSD) that was selected to define TSS removal for the Stormceptor design.		
Fine Distribution		
Particle Diameter (microns)	Distribution %	Specific Gravity
20.0	20.0	1.30
60.0	20.0	1.80
150.0	20.0	2.20
400.0	20.0	2.65
2000.0	20.0	2.65

Site Name		Sub Area 1: parking area	
Site Details			
Drainage Area		Infiltration Parameters	
Total Area (acres)	0.7	Horton's equation is used to estimate infiltration	
Imperviousness %	98.0	Max. Infiltration Rate (in/hr)	2.44
Surface Characteristics		Min. Infiltration Rate (in/hr)	0.4
Width (ft)	349.00	Decay Rate (1/sec)	0.00055
Slope %	2	Regeneration Rate (1/sec)	0.01
Impervious Depression Storage (in)	0.02	Evaporation	
Pervious Depression Storage (in)	0.2	Daily Evaporation Rate (in/day)	0.1
Impervious Manning's n	0.015	Dry Weather Flow	
Pervious Manning's n	0.25	Dry Weather Flow (cfs)	0
Maintenance Frequency		Winter Months	
Maintenance Frequency (months) >	12	Winter Infiltration	0
TSS Loading Parameters			
TSS Loading Function			
Buildup/Wash-off Parameters		TSS Availability Parameters	
Target Event Mean Conc. (EMC) mg/L		Availability Constant A	
Exponential Buildup Power		Availability Factor B	
Exponential Washoff Exponent		Availability Exponent C	
		Min. Particle Size Affected by Availability (micron)	

Cumulative Runoff Volume by Runoff Rate			
Runoff Rate (cfs)	Runoff Volume (ft³)	Volume Over (ft³)	Cumulative Runoff Volume (%)
0.035	634286	1280047	33.1
0.141	1179276	734993	61.6
0.318	1499172	415111	78.3
0.565	1683445	230843	87.9
0.883	1785013	129281	93.2
1.271	1841418	72874	96.2
1.730	1872413	41882	97.8
2.260	1890037	24258	98.7
2.860	1900947	13347	99.3
3.531	1907392	6902	99.6
4.273	1911387	2908	99.8
5.085	1913979	318	100.0
5.968	1914297	0	100.0
6.922	1914297	0	100.0

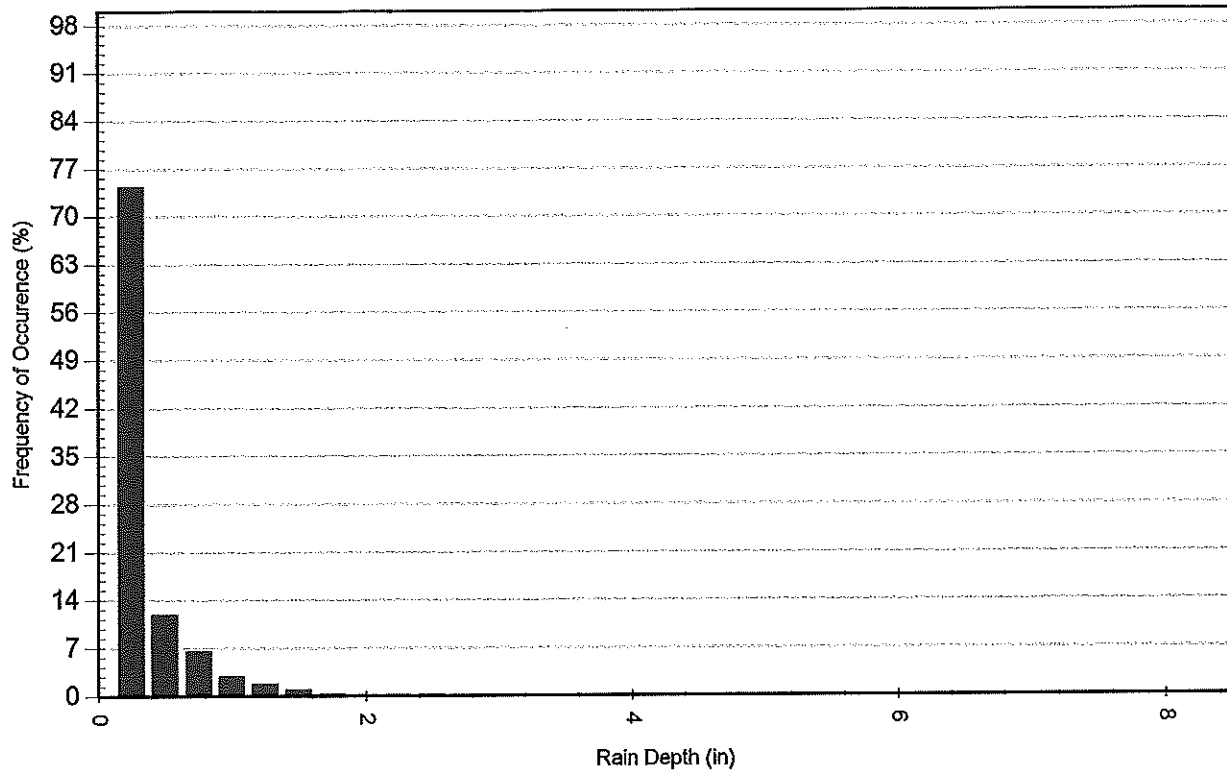
Cumulative Runoff Volume by Runoff Rate

For area: 0.7(ac), imperviousness: 98.0%, rainfall station: PITTSFIELD WB AIRPORT

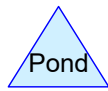
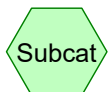
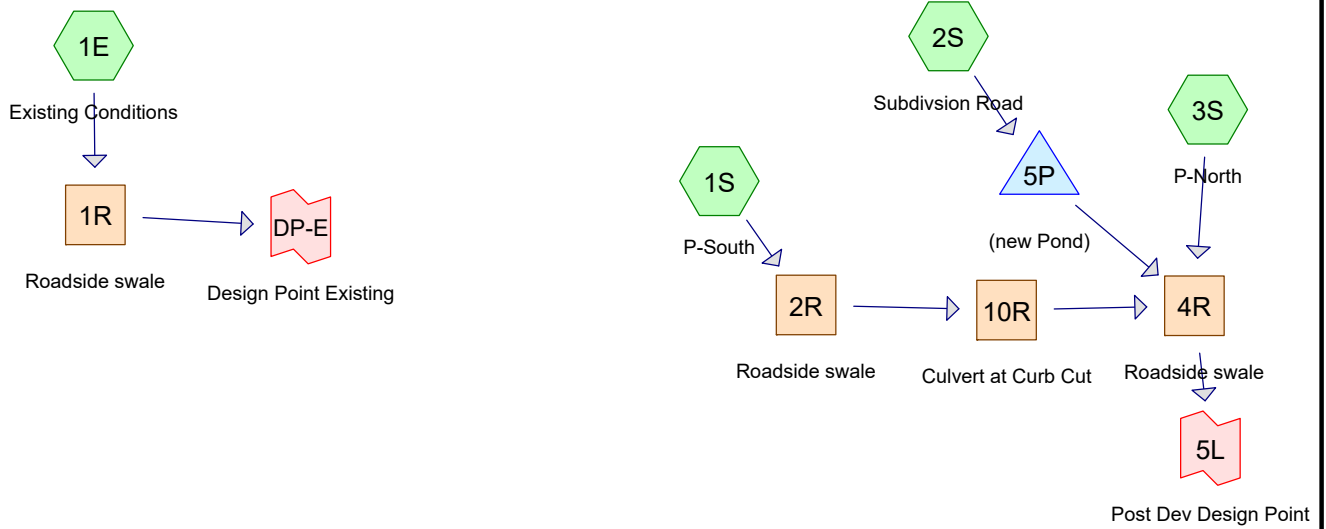


Rainfall Event Analysis				
Rainfall Depth (in)	No. of Events	Percentage of Total Events (%)	Total Volume (in)	Percentage of Annual Volume (%)
0.25	2855	74.4	193	22.8
0.50	460	12.0	166	19.6
0.75	254	6.6	156	18.4
1.00	117	3.0	103	12.1
1.25	69	1.8	77	9.1
1.50	38	1.0	52	6.1
1.75	13	0.3	21	2.4
2.00	8	0.2	15	1.7
2.25	6	0.2	13	1.5
2.50	9	0.2	21	2.5
2.75	3	0.1	8	1.0
3.00	3	0.1	9	1.0
3.25	1	0.0	3	0.4
3.50	1	0.0	3	0.4
3.75	0	0.0	0	0.0
4.00	0	0.0	0	0.0
4.25	0	0.0	0	0.0
4.50	0	0.0	0	0.0
4.75	0	0.0	0	0.0
5.00	0	0.0	0	0.0
5.25	0	0.0	0	0.0
5.50	0	0.0	0	0.0
5.75	0	0.0	0	0.0
6.00	0	0.0	0	0.0
6.25	0	0.0	0	0.0
6.50	0	0.0	0	0.0
6.75	0	0.0	0	0.0
7.00	0	0.0	0	0.0
7.25	0	0.0	0	0.0
7.50	0	0.0	0	0.0
7.75	0	0.0	0	0.0
8.00	0	0.0	0	0.0
8.25	0	0.0	0	0.0
8.25	1	0.0	8	1.0

Frequency of Occurrence by Rainfall Depths



For Stormceptor Specifications and Drawings Please Visit:
<https://www.conteches.com/technical-guides/search?filter=1WBC005EYX>



SWM Calcs

Prepared by SK Design Group, Inc

Printed 12/1/2023

HydroCAD® 10.00-26 s/n 01869 © 2020 HydroCAD Software Solutions LLC

Page 2

Area Listing (all nodes)

Area (acres)	CN	Description (subcatchment-numbers)
4.000	77	2 acre lots, 12% imp, HSG C (1S, 3S)
15.756	74	>75% Grass cover, Good, HSG C (1E, 1S, 2S, 3S)
0.073	98	Driveway from Cul de Sac (1S)
0.073	98	Gymnasium Roof (1E)
0.925	98	Paved parking, HSG C (2S, 3S)
0.052	98	Roof Main house (1S)
0.289	98	Roofs, HSG C (2S, 3S)
0.158	98	Rooftop area Main house (1E)
7.206	70	Woods, Good, HSG C (1E, 1S, 2S, 3S)
0.769	98	existing looped driveway (1E)

SWM Calcs

Prepared by SK Design Group, Inc

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Type III 24-hr 100 YR Rainfall=7.29"

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Page 3

Summary for Subcatchment 1E: Existing Conditions

Runoff = 65.44 cfs @ 12.12 hrs, Volume= 4.804 af, Depth> 4.01"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=7.29"

	Area (sf)	CN	Description
*	6,875	98	Rooftop area Main house
*	3,200	98	Gymnasium Roof
*	33,500	98	exisitng looped driveway
	183,775	70	Woods, Good, HSG C
	398,380	74	>75% Grass cover, Good, HSG C
	625,730	74	Weighted Average
	582,155		93.04% Pervious Area
	43,575		6.96% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.6	50	0.0240	0.15		Sheet Flow, Grass: Short n= 0.150 P2= 2.81"
0.4	70	0.0420	3.07		Shallow Concentrated Flow, Grassed waterway Grassed Waterway Kv= 15.0 fps
2.5	1,045	0.0100	7.07	56.55	Parabolic Channel, W=6.00' D=2.00' Area=8.0 sf Perim=7.5' n= 0.022
8.5	1,165	Total			

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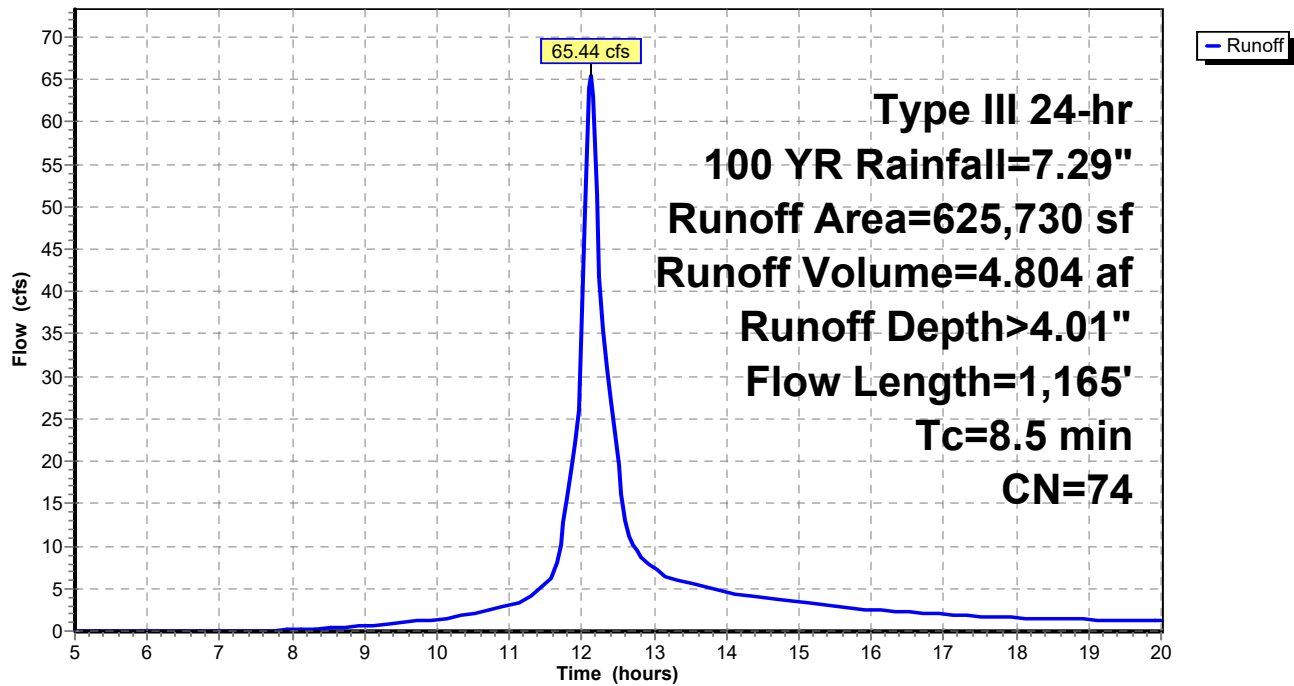
Type III 24-hr 100 YR Rainfall=7.29"

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Page 4

Subcatchment 1E: Existing Conditions

Hydrograph



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Type III 24-hr 100 YR Rainfall=7.29"

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Page 5

Summary for Subcatchment 1S: P-South

Runoff = 21.73 cfs @ 12.15 hrs, Volume= 1.719 af, Depth> 4.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=7.29"

Area (sf)	CN	Description
87,120	77	2 acre lots, 12% imp, HSG C
* 2,275	98	Roof Main house
29,830	70	Woods, Good, HSG C
* 3,200	98	Driveway from Cul de Sac
95,805	74	>75% Grass cover, Good, HSG C
218,230	75	Weighted Average
202,301		92.70% Pervious Area
15,929		7.30% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
6.0	50	0.0200	0.14		Sheet Flow, Grass: Short n= 0.150 P2= 2.81"
2.6	500	0.0460	3.22		Shallow Concentrated Flow, Overland flow along southerly driv Grassed Waterway Kv= 15.0 fps
2.4	1,000	0.0100	7.07	56.55	Parabolic Channel, W=6.00' D=2.00' Area=8.0 sf Perim=7.5' n= 0.022
11.0	1,550	Total			

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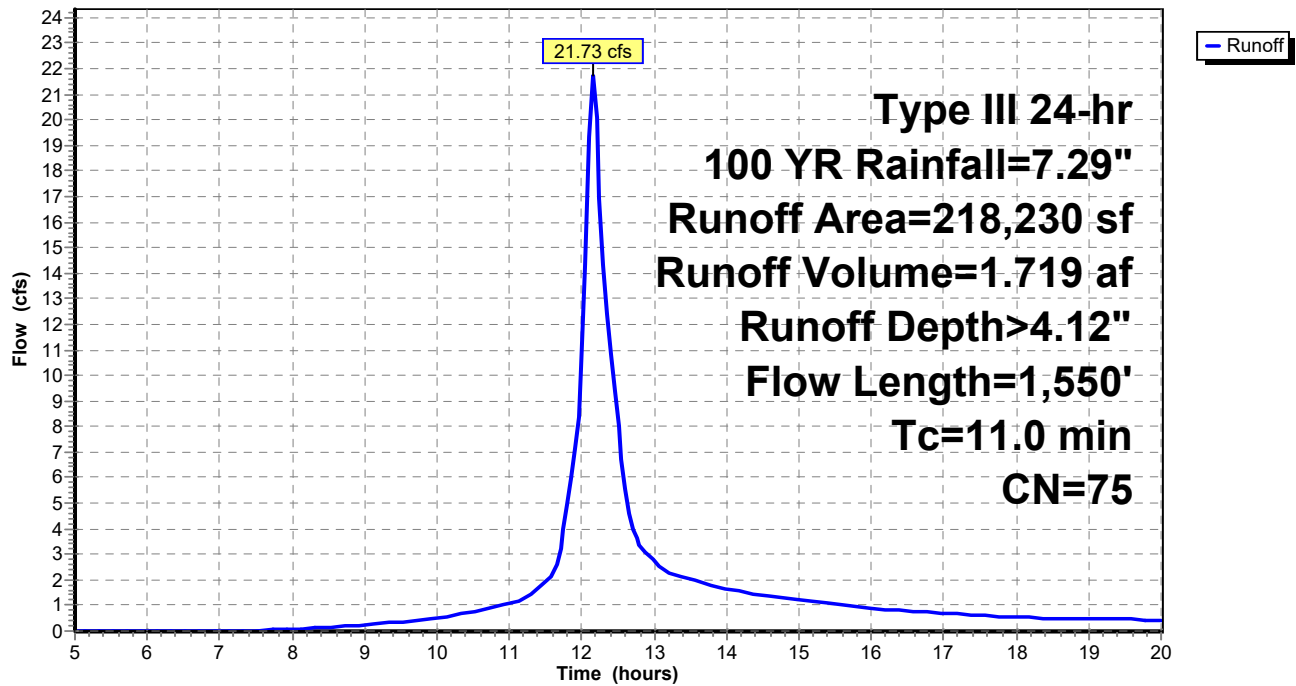
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Page 6

Subcatchment 1S: P-South

Hydrograph



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Type III 24-hr 100 YR Rainfall=7.29"

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Page 7

Summary for Subcatchment 2S: Subdivsion Road

Runoff = 11.47 cfs @ 12.06 hrs, Volume= 0.728 af, Depth> 4.67"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=7.29"

Area (sf)	CN	Description
15,360	98	Paved parking, HSG C
26,115	74	>75% Grass cover, Good, HSG C
8,825	98	Roofs, HSG C
31,200	70	Woods, Good, HSG C
81,500	80	Weighted Average
57,315		70.33% Pervious Area
24,185		29.67% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
1.0	50	0.0100	0.85		Sheet Flow, Roof Smooth surfaces n= 0.011 P2= 2.81"
0.6	130	0.0500	3.35		Shallow Concentrated Flow, Lawn Grassed Waterway Kv= 15.0 fps
0.5	100	0.0300	3.52		Shallow Concentrated Flow, Paved Kv= 20.3 fps
0.4	300	0.0800	13.90	10.92	Pipe Channel, 12.0" Round Area= 0.8 sf Perim= 3.1' r= 0.25' n= 0.012
1.1	450	0.0100	7.07	56.55	Parabolic Channel, W=6.00' D=2.00' Area=8.0 sf Perim=7.5' n= 0.022
3.6	1,030	Total			

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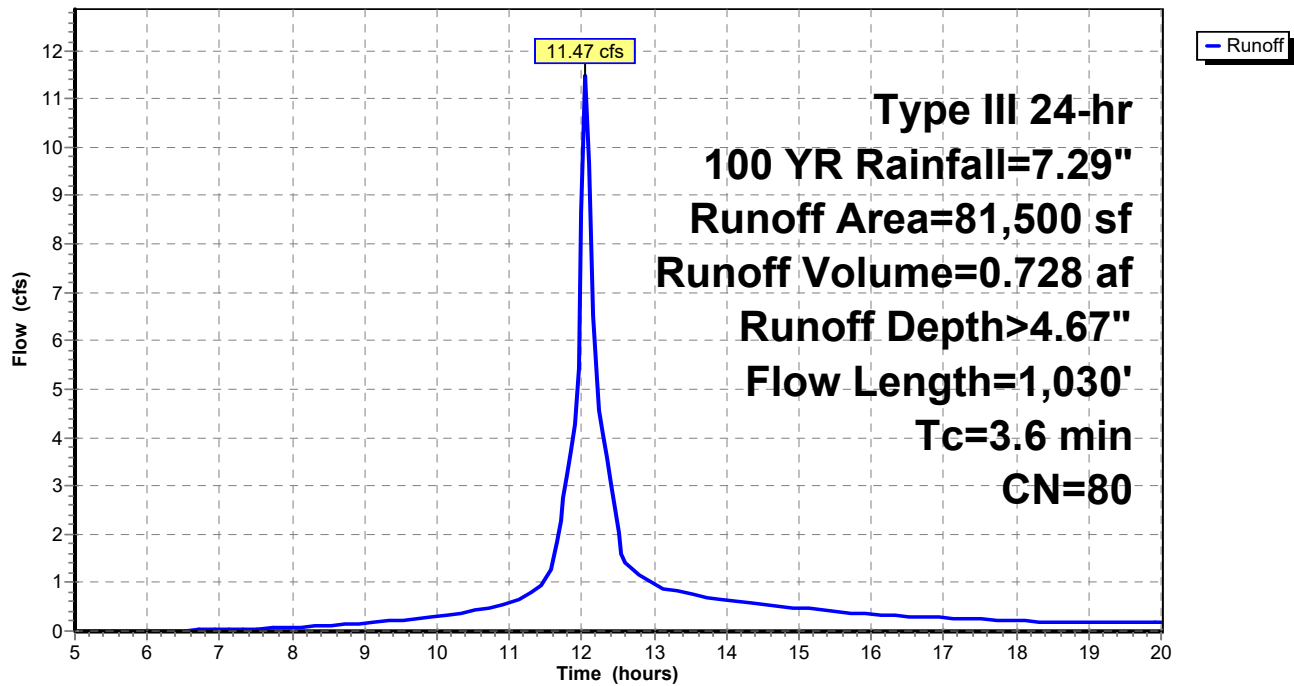
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Page 8

Subcatchment 2S: Subdivision Road

Hydrograph



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Type III 24-hr 100 YR Rainfall=7.29"

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Page 9

Summary for Subcatchment 3S: P-North

Runoff = 41.56 cfs @ 12.09 hrs, Volume= 2.840 af, Depth> 4.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Type III 24-hr 100 YR Rainfall=7.29"

Area (sf)	CN	Description
87,120	77	2 acre lots, 12% imp, HSG C
3,760	98	Roofs, HSG C
69,088	70	Woods, Good, HSG C
24,925	98	Paved parking, HSG C
166,032	74	>75% Grass cover, Good, HSG C
350,925	76	Weighted Average
311,786		88.85% Pervious Area
39,139		11.15% Impervious Area

Tc (min)	Length (feet)	Slope (ft/ft)	Velocity (ft/sec)	Capacity (cfs)	Description
5.6	50	0.0240	0.15		Sheet Flow, Grass: Short n= 0.150 P2= 2.81"
0.4	70	0.0420	3.07		Shallow Concentrated Flow, Grass along driveway Grassed Waterway Kv= 15.0 fps
0.1	50	0.0100	7.07	56.55	Parabolic Channel, W=6.00' D=2.00' Area=8.0 sf Perim=7.5' n= 0.022
6.1	170	Total			

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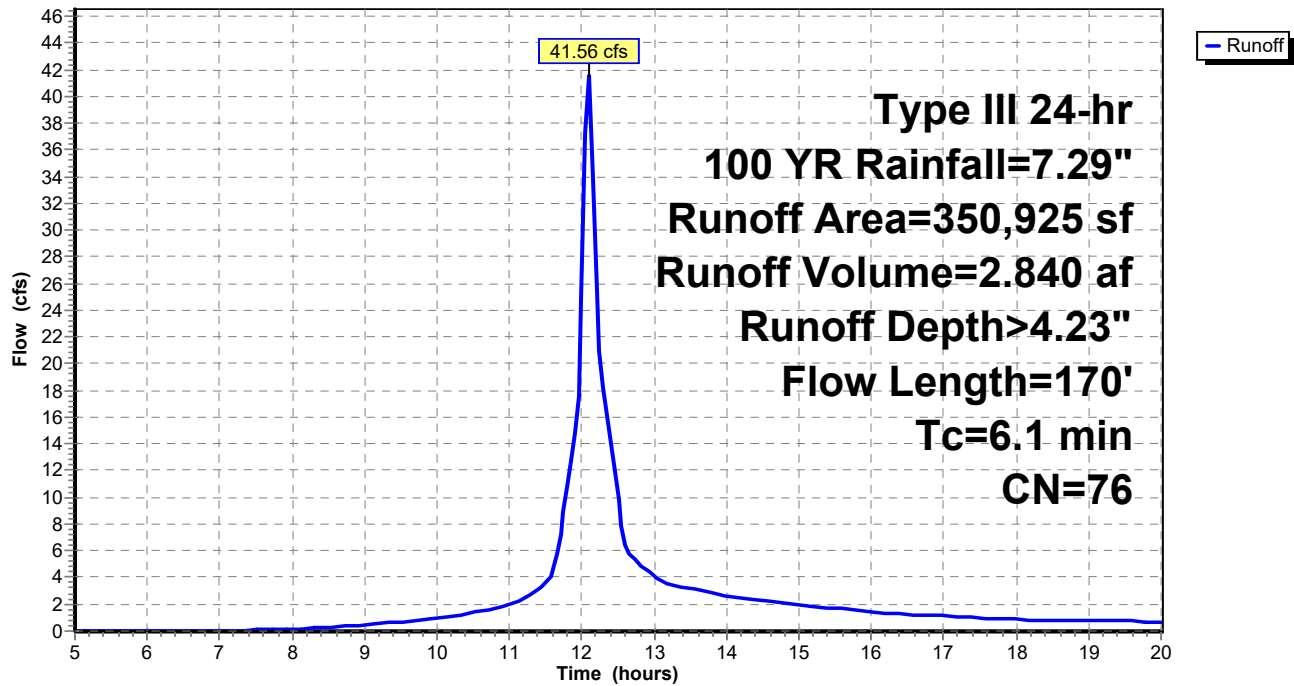
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Page 10

Subcatchment 3S: P-North

Hydrograph



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Type III 24-hr 100 YR Rainfall=7.29"

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Page 11

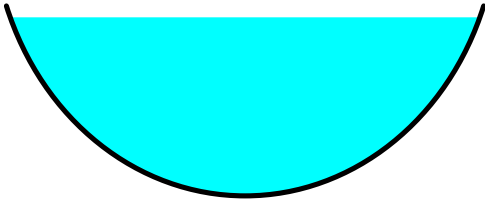
Summary for Reach 1R: Roadside swale

Inflow Area = 14.365 ac, 6.96% Impervious, Inflow Depth > 4.01" for 100 YR event
Inflow = 65.44 cfs @ 12.12 hrs, Volume= 4.804 af
Outflow = 61.72 cfs @ 12.19 hrs, Volume= 4.788 af, Atten= 6%, Lag= 4.4 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Max. Velocity= 7.70 fps, Min. Travel Time= 2.3 min
Avg. Velocity= 3.09 fps, Avg. Travel Time= 5.6 min

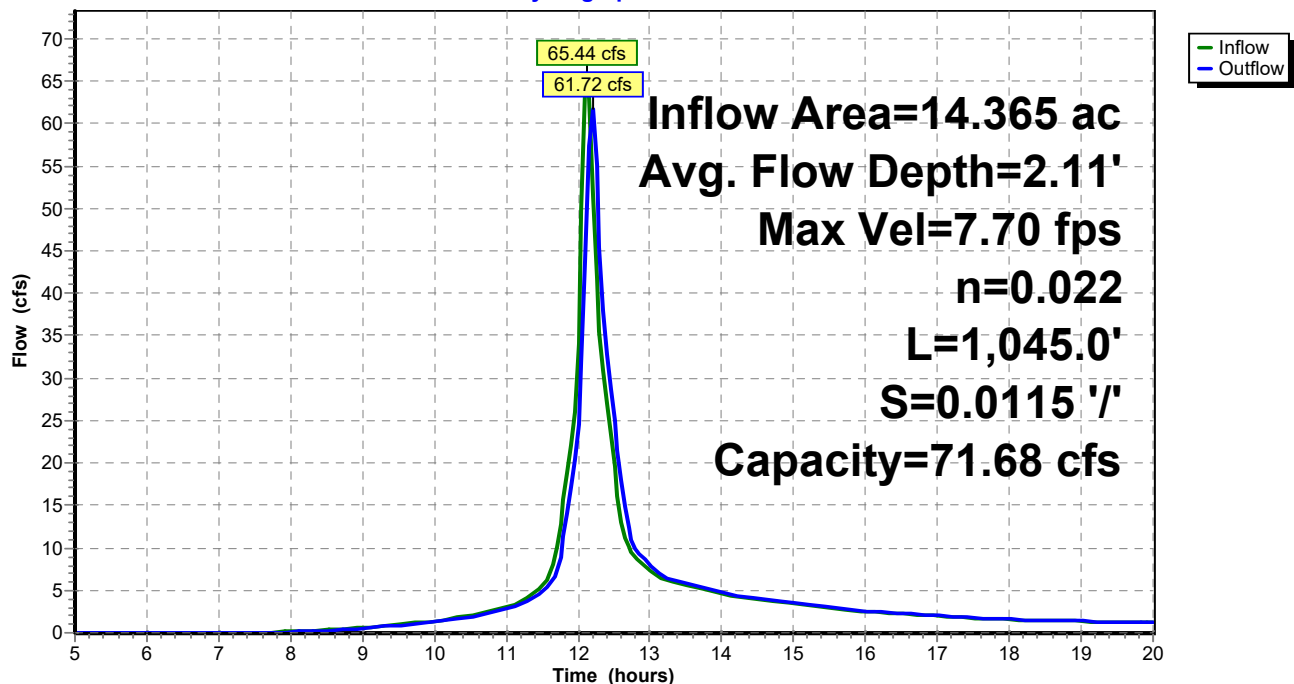
Peak Storage= 8,558 cf @ 12.15 hrs
Average Depth at Peak Storage= 2.11'
Bank-Full Depth= 2.25' Flow Area= 9.0 sf, Capacity= 71.68 cfs

6.00' x 2.25' deep Parabolic Channel, n= 0.022 Earth, clean & straight
Length= 1,045.0' Slope= 0.0115 '/'
Inlet Invert= 1,046.00', Outlet Invert= 1,034.00'



Reach 1R: Roadside swale

Hydrograph



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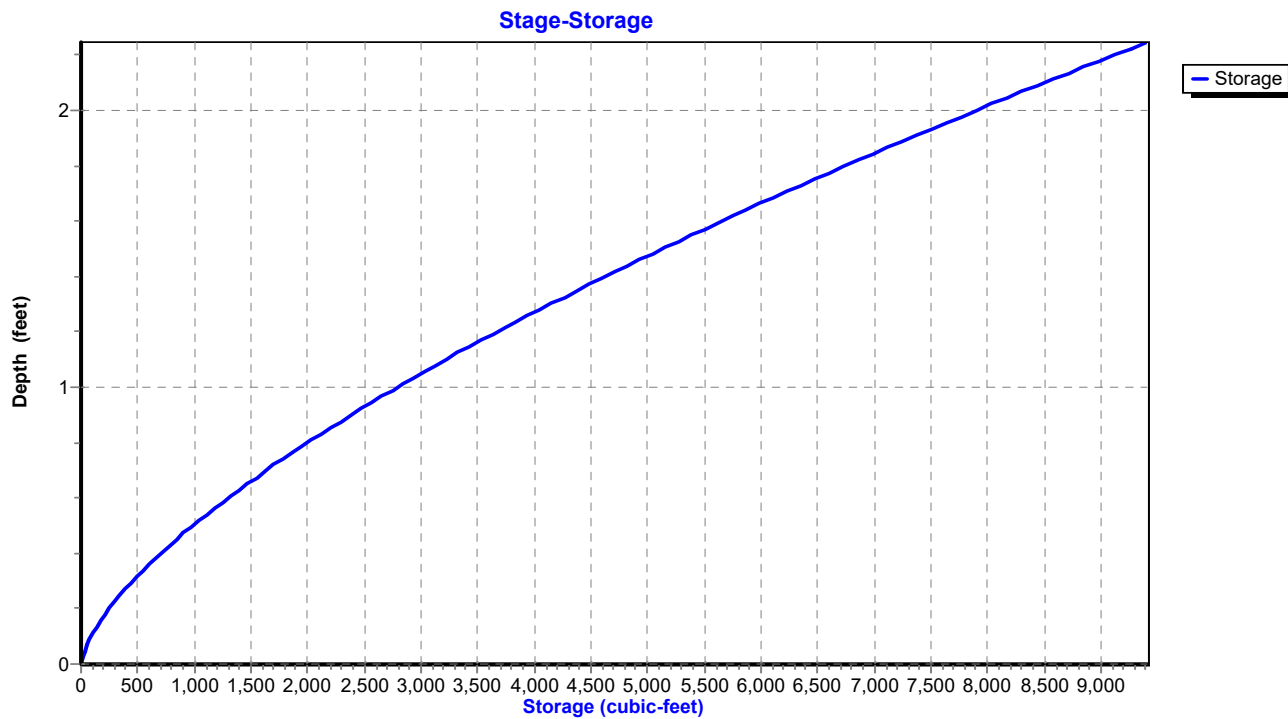
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Page 12

Reach 1R: Roadside swale



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Type III 24-hr 100 YR Rainfall=7.29"

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Page 13

Stage-Area-Storage for Reach 1R: Roadside swale

Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)	Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)
1,046.00	0.0	0	1,046.52	1.0	1,045
1,046.01	0.0	4	1,046.53	1.0	1,075
1,046.02	0.0	8	1,046.54	1.1	1,106
1,046.03	0.0	15	1,046.55	1.1	1,137
1,046.04	0.0	23	1,046.56	1.1	1,168
1,046.05	0.0	32	1,046.57	1.1	1,199
1,046.06	0.0	41	1,046.58	1.2	1,231
1,046.07	0.0	52	1,046.59	1.2	1,263
1,046.08	0.1	64	1,046.60	1.2	1,295
1,046.09	0.1	75	1,046.61	1.3	1,328
1,046.10	0.1	89	1,046.62	1.3	1,361
1,046.11	0.1	102	1,046.63	1.3	1,393
1,046.12	0.1	116	1,046.64	1.4	1,427
1,046.13	0.1	131	1,046.65	1.4	1,460
1,046.14	0.1	146	1,046.66	1.4	1,494
1,046.15	0.2	162	1,046.67	1.5	1,528
1,046.16	0.2	178	1,046.68	1.5	1,563
1,046.17	0.2	196	1,046.69	1.5	1,597
1,046.18	0.2	213	1,046.70	1.6	1,632
1,046.19	0.2	231	1,046.71	1.6	1,667
1,046.20	0.2	249	1,046.72	1.6	1,702
1,046.21	0.3	268	1,046.73	1.7	1,738
1,046.22	0.3	288	1,046.74	1.7	1,774
1,046.23	0.3	308	1,046.75	1.7	1,810
1,046.24	0.3	328	1,046.76	1.8	1,846
1,046.25	0.3	348	1,046.77	1.8	1,883
1,046.26	0.4	370	1,046.78	1.8	1,920
1,046.27	0.4	391	1,046.79	1.9	1,957
1,046.28	0.4	413	1,046.80	1.9	1,994
1,046.29	0.4	435	1,046.81	1.9	2,031
1,046.30	0.4	458	1,046.82	2.0	2,069
1,046.31	0.5	481	1,046.83	2.0	2,107
1,046.32	0.5	505	1,046.84	2.1	2,146
1,046.33	0.5	528	1,046.85	2.1	2,184
1,046.34	0.5	553	1,046.86	2.1	2,223
1,046.35	0.6	577	1,046.87	2.2	2,261
1,046.36	0.6	602	1,046.88	2.2	2,300
1,046.37	0.6	627	1,046.89	2.2	2,340
1,046.38	0.6	653	1,046.90	2.3	2,379
1,046.39	0.6	679	1,046.91	2.3	2,419
1,046.40	0.7	705	1,046.92	2.4	2,459
1,046.41	0.7	732	1,046.93	2.4	2,499
1,046.42	0.7	759	1,046.94	2.4	2,540
1,046.43	0.8	786	1,046.95	2.5	2,580
1,046.44	0.8	814	1,046.96	2.5	2,621
1,046.45	0.8	841	1,046.97	2.5	2,662
1,046.46	0.8	870	1,046.98	2.6	2,704
1,046.47	0.9	898	1,046.99	2.6	2,745
1,046.48	0.9	927	1,047.00	2.7	2,787
1,046.49	0.9	956	1,047.01	2.7	2,829
1,046.50	0.9	985	1,047.02	2.7	2,871
1,046.51	1.0	1,015	1,047.03	2.8	2,913

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Type III 24-hr 100 YR Rainfall=7.29"

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Page 14

Stage-Area-Storage for Reach 1R: Roadside swale (continued)

Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)	Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)
1,047.04	2.8	2,956	1,047.56	5.2	5,430
1,047.05	2.9	2,998	1,047.57	5.2	5,482
1,047.06	2.9	3,041	1,047.58	5.3	5,534
1,047.07	3.0	3,084	1,047.59	5.3	5,587
1,047.08	3.0	3,128	1,047.60	5.4	5,640
1,047.09	3.0	3,171	1,047.61	5.4	5,693
1,047.10	3.1	3,215	1,047.62	5.5	5,746
1,047.11	3.1	3,259	1,047.63	5.5	5,799
1,047.12	3.2	3,303	1,047.64	5.6	5,853
1,047.13	3.2	3,347	1,047.65	5.7	5,906
1,047.14	3.2	3,392	1,047.66	5.7	5,960
1,047.15	3.3	3,437	1,047.67	5.8	6,014
1,047.16	3.3	3,482	1,047.68	5.8	6,068
1,047.17	3.4	3,527	1,047.69	5.9	6,122
1,047.18	3.4	3,572	1,047.70	5.9	6,177
1,047.19	3.5	3,618	1,047.71	6.0	6,231
1,047.20	3.5	3,663	1,047.72	6.0	6,286
1,047.21	3.5	3,709	1,047.73	6.1	6,341
1,047.22	3.6	3,755	1,047.74	6.1	6,396
1,047.23	3.6	3,801	1,047.75	6.2	6,451
1,047.24	3.7	3,848	1,047.76	6.2	6,507
1,047.25	3.7	3,895	1,047.77	6.3	6,562
1,047.26	3.8	3,941	1,047.78	6.3	6,618
1,047.27	3.8	3,988	1,047.79	6.4	6,674
1,047.28	3.9	4,036	1,047.80	6.4	6,730
1,047.29	3.9	4,083	1,047.81	6.5	6,786
1,047.30	4.0	4,131	1,047.82	6.5	6,842
1,047.31	4.0	4,178	1,047.83	6.6	6,899
1,047.32	4.0	4,226	1,047.84	6.7	6,955
1,047.33	4.1	4,274	1,047.85	6.7	7,012
1,047.34	4.1	4,323	1,047.86	6.8	7,069
1,047.35	4.2	4,371	1,047.87	6.8	7,126
1,047.36	4.2	4,420	1,047.88	6.9	7,183
1,047.37	4.3	4,469	1,047.89	6.9	7,241
1,047.38	4.3	4,518	1,047.90	7.0	7,298
1,047.39	4.4	4,567	1,047.91	7.0	7,356
1,047.40	4.4	4,616	1,047.92	7.1	7,414
1,047.41	4.5	4,666	1,047.93	7.2	7,472
1,047.42	4.5	4,715	1,047.94	7.2	7,530
1,047.43	4.6	4,765	1,047.95	7.3	7,588
1,047.44	4.6	4,815	1,047.96	7.3	7,647
1,047.45	4.7	4,866	1,047.97	7.4	7,705
1,047.46	4.7	4,916	1,047.98	7.4	7,764
1,047.47	4.8	4,967	1,047.99	7.5	7,823
1,047.48	4.8	5,017	1,048.00	7.5	7,882
1,047.49	4.9	5,068	1,048.01	7.6	7,941
1,047.50	4.9	5,120	1,048.02	7.7	8,000
1,047.51	4.9	5,171	1,048.03	7.7	8,060
1,047.52	5.0	5,222	1,048.04	7.8	8,120
1,047.53	5.0	5,274	1,048.05	7.8	8,179
1,047.54	5.1	5,326	1,048.06	7.9	8,239
1,047.55	5.1	5,378	1,048.07	7.9	8,299

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Page 15

Stage-Area-Storage for Reach 1R: Roadside swale (continued)

Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)
1,048.08	8.0	8,360
1,048.09	8.1	8,420
1,048.10	8.1	8,480
1,048.11	8.2	8,541
1,048.12	8.2	8,602
1,048.13	8.3	8,663
1,048.14	8.3	8,724
1,048.15	8.4	8,785
1,048.16	8.5	8,846
1,048.17	8.5	8,908
1,048.18	8.6	8,970
1,048.19	8.6	9,031
1,048.20	8.7	9,093
1,048.21	8.8	9,155
1,048.22	8.8	9,218
1,048.23	8.9	9,280
1,048.24	8.9	9,342
1,048.25	9.0	9,405

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Page 16

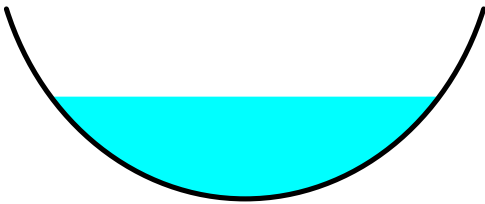
Summary for Reach 2R: Roadside swale

Inflow Area = 5.010 ac, 7.30% Impervious, Inflow Depth > 4.12" for 100 YR event
Inflow = 21.73 cfs @ 12.15 hrs, Volume= 1.719 af
Outflow = 21.01 cfs @ 12.20 hrs, Volume= 1.716 af, Atten= 3%, Lag= 2.4 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Max. Velocity= 6.04 fps, Min. Travel Time= 1.2 min
Avg. Velocity= 2.40 fps, Avg. Travel Time= 3.1 min

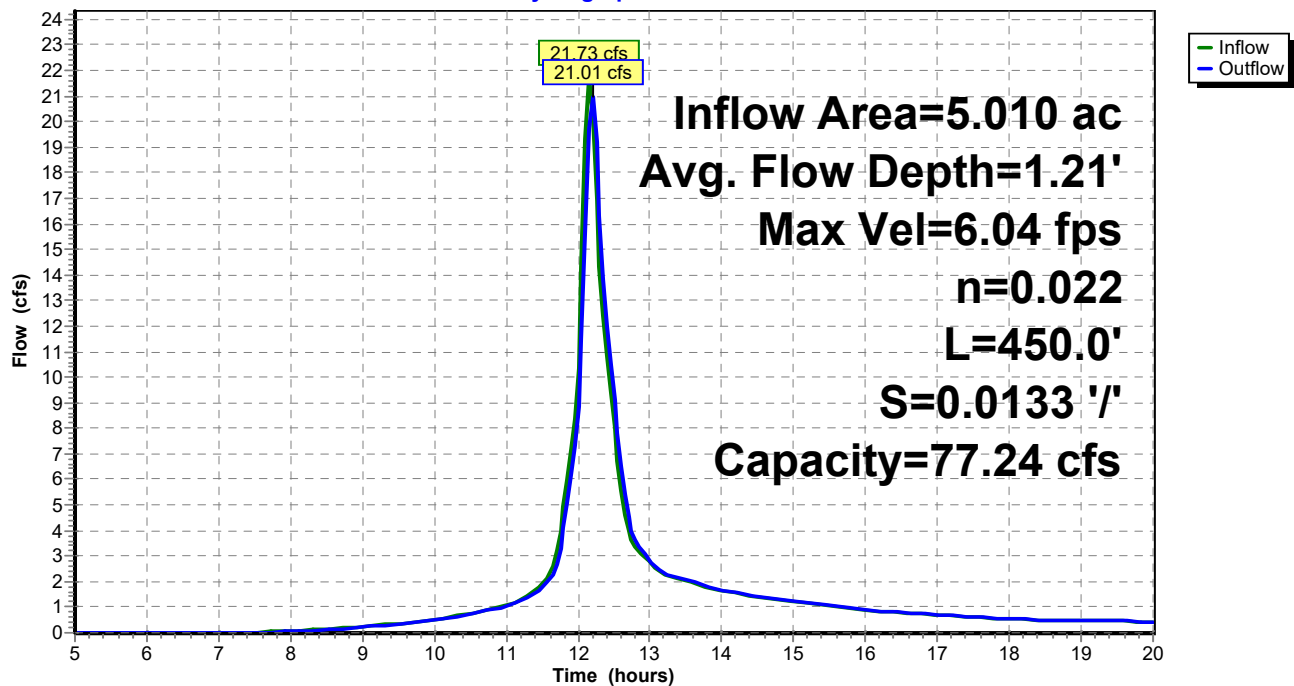
Peak Storage= 1,597 cf @ 12.17 hrs
Average Depth at Peak Storage= 1.21'
Bank-Full Depth= 2.25' Flow Area= 9.0 sf, Capacity= 77.24 cfs

6.00' x 2.25' deep Parabolic Channel, n= 0.022 Earth, clean & straight
Length= 450.0' Slope= 0.0133 '/'
Inlet Invert= 1,046.00', Outlet Invert= 1,040.00'



Reach 2R: Roadside swale

Hydrograph



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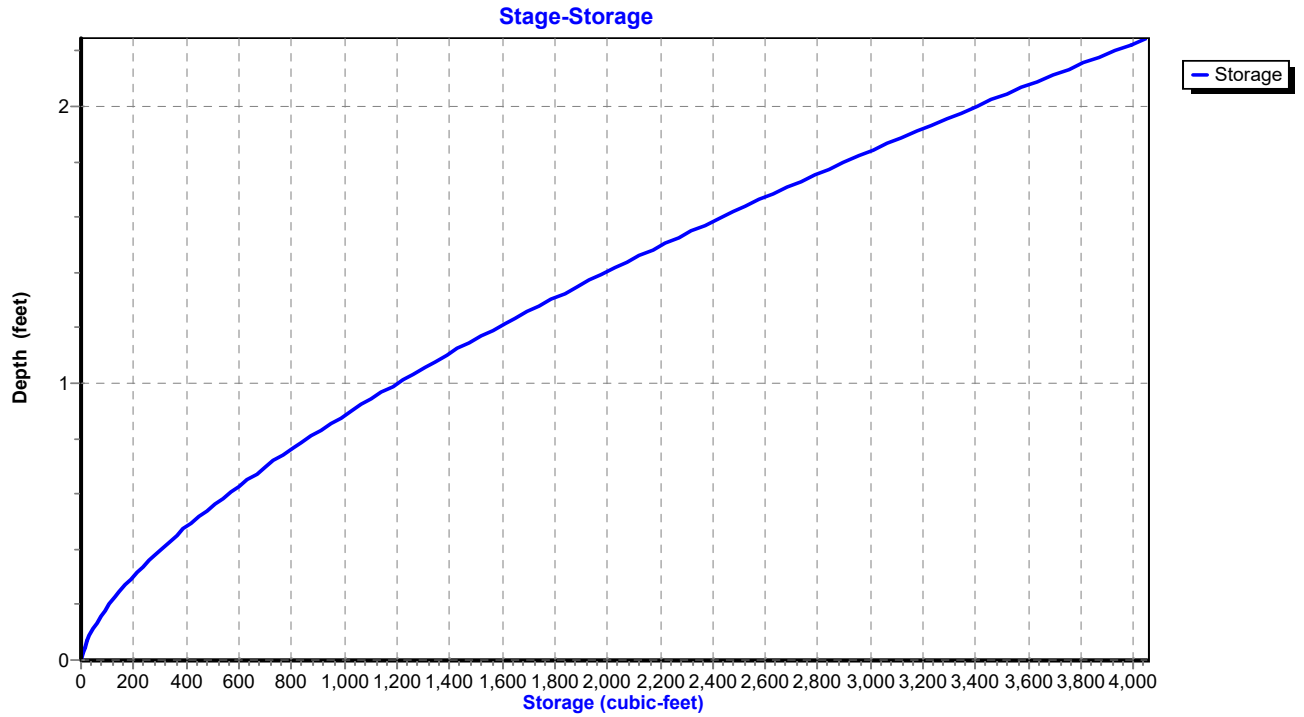
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Type III 24-hr 100 YR Rainfall=7.29"

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Page 17

Reach 2R: Roadside swale



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Type III 24-hr 100 YR Rainfall=7.29"

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Page 18

Stage-Area-Storage for Reach 2R: Roadside swale

Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)	Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)
1,046.00	0.0	0	1,046.52	1.0	450
1,046.01	0.0	2	1,046.53	1.0	463
1,046.02	0.0	4	1,046.54	1.1	476
1,046.03	0.0	7	1,046.55	1.1	490
1,046.04	0.0	10	1,046.56	1.1	503
1,046.05	0.0	14	1,046.57	1.1	516
1,046.06	0.0	18	1,046.58	1.2	530
1,046.07	0.0	22	1,046.59	1.2	544
1,046.08	0.1	27	1,046.60	1.2	558
1,046.09	0.1	32	1,046.61	1.3	572
1,046.10	0.1	38	1,046.62	1.3	586
1,046.11	0.1	44	1,046.63	1.3	600
1,046.12	0.1	50	1,046.64	1.4	614
1,046.13	0.1	56	1,046.65	1.4	629
1,046.14	0.1	63	1,046.66	1.4	643
1,046.15	0.2	70	1,046.67	1.5	658
1,046.16	0.2	77	1,046.68	1.5	673
1,046.17	0.2	84	1,046.69	1.5	688
1,046.18	0.2	92	1,046.70	1.6	703
1,046.19	0.2	100	1,046.71	1.6	718
1,046.20	0.2	107	1,046.72	1.6	733
1,046.21	0.3	116	1,046.73	1.7	749
1,046.22	0.3	124	1,046.74	1.7	764
1,046.23	0.3	132	1,046.75	1.7	779
1,046.24	0.3	141	1,046.76	1.8	795
1,046.25	0.3	150	1,046.77	1.8	811
1,046.26	0.4	159	1,046.78	1.8	827
1,046.27	0.4	168	1,046.79	1.9	843
1,046.28	0.4	178	1,046.80	1.9	859
1,046.29	0.4	187	1,046.81	1.9	875
1,046.30	0.4	197	1,046.82	2.0	891
1,046.31	0.5	207	1,046.83	2.0	907
1,046.32	0.5	217	1,046.84	2.1	924
1,046.33	0.5	228	1,046.85	2.1	940
1,046.34	0.5	238	1,046.86	2.1	957
1,046.35	0.6	249	1,046.87	2.2	974
1,046.36	0.6	259	1,046.88	2.2	991
1,046.37	0.6	270	1,046.89	2.2	1,008
1,046.38	0.6	281	1,046.90	2.3	1,025
1,046.39	0.6	292	1,046.91	2.3	1,042
1,046.40	0.7	304	1,046.92	2.4	1,059
1,046.41	0.7	315	1,046.93	2.4	1,076
1,046.42	0.7	327	1,046.94	2.4	1,094
1,046.43	0.8	338	1,046.95	2.5	1,111
1,046.44	0.8	350	1,046.96	2.5	1,129
1,046.45	0.8	362	1,046.97	2.5	1,146
1,046.46	0.8	374	1,046.98	2.6	1,164
1,046.47	0.9	387	1,046.99	2.6	1,182
1,046.48	0.9	399	1,047.00	2.7	1,200
1,046.49	0.9	412	1,047.01	2.7	1,218
1,046.50	0.9	424	1,047.02	2.7	1,236
1,046.51	1.0	437	1,047.03	2.8	1,254

SWM Calcs

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Type III 24-hr 100 YR Rainfall=7.29"

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Page 19

Stage-Area-Storage for Reach 2R: Roadside swale (continued)

Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)	Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)
1,047.04	2.8	1,273	1,047.56	5.2	2,338
1,047.05	2.9	1,291	1,047.57	5.2	2,361
1,047.06	2.9	1,310	1,047.58	5.3	2,383
1,047.07	3.0	1,328	1,047.59	5.3	2,406
1,047.08	3.0	1,347	1,047.60	5.4	2,429
1,047.09	3.0	1,366	1,047.61	5.4	2,451
1,047.10	3.1	1,384	1,047.62	5.5	2,474
1,047.11	3.1	1,403	1,047.63	5.5	2,497
1,047.12	3.2	1,422	1,047.64	5.6	2,520
1,047.13	3.2	1,441	1,047.65	5.7	2,543
1,047.14	3.2	1,461	1,047.66	5.7	2,567
1,047.15	3.3	1,480	1,047.67	5.8	2,590
1,047.16	3.3	1,499	1,047.68	5.8	2,613
1,047.17	3.4	1,519	1,047.69	5.9	2,636
1,047.18	3.4	1,538	1,047.70	5.9	2,660
1,047.19	3.5	1,558	1,047.71	6.0	2,683
1,047.20	3.5	1,577	1,047.72	6.0	2,707
1,047.21	3.5	1,597	1,047.73	6.1	2,731
1,047.22	3.6	1,617	1,047.74	6.1	2,754
1,047.23	3.6	1,637	1,047.75	6.2	2,778
1,047.24	3.7	1,657	1,047.76	6.2	2,802
1,047.25	3.7	1,677	1,047.77	6.3	2,826
1,047.26	3.8	1,697	1,047.78	6.3	2,850
1,047.27	3.8	1,718	1,047.79	6.4	2,874
1,047.28	3.9	1,738	1,047.80	6.4	2,898
1,047.29	3.9	1,758	1,047.81	6.5	2,922
1,047.30	4.0	1,779	1,047.82	6.5	2,946
1,047.31	4.0	1,799	1,047.83	6.6	2,971
1,047.32	4.0	1,820	1,047.84	6.7	2,995
1,047.33	4.1	1,841	1,047.85	6.7	3,020
1,047.34	4.1	1,861	1,047.86	6.8	3,044
1,047.35	4.2	1,882	1,047.87	6.8	3,069
1,047.36	4.2	1,903	1,047.88	6.9	3,093
1,047.37	4.3	1,924	1,047.89	6.9	3,118
1,047.38	4.3	1,945	1,047.90	7.0	3,143
1,047.39	4.4	1,967	1,047.91	7.0	3,168
1,047.40	4.4	1,988	1,047.92	7.1	3,193
1,047.41	4.5	2,009	1,047.93	7.2	3,218
1,047.42	4.5	2,031	1,047.94	7.2	3,243
1,047.43	4.6	2,052	1,047.95	7.3	3,268
1,047.44	4.6	2,074	1,047.96	7.3	3,293
1,047.45	4.7	2,095	1,047.97	7.4	3,318
1,047.46	4.7	2,117	1,047.98	7.4	3,343
1,047.47	4.8	2,139	1,047.99	7.5	3,369
1,047.48	4.8	2,161	1,048.00	7.5	3,394
1,047.49	4.9	2,183	1,048.01	7.6	3,420
1,047.50	4.9	2,205	1,048.02	7.7	3,445
1,047.51	4.9	2,227	1,048.03	7.7	3,471
1,047.52	5.0	2,249	1,048.04	7.8	3,496
1,047.53	5.0	2,271	1,048.05	7.8	3,522
1,047.54	5.1	2,293	1,048.06	7.9	3,548
1,047.55	5.1	2,316	1,048.07	7.9	3,574

SWM Calcs

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Type III 24-hr 100 YR Rainfall=7.29"

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Page 20

Stage-Area-Storage for Reach 2R: Roadside swale (continued)

Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)
1,048.08	8.0	3,600
1,048.09	8.1	3,626
1,048.10	8.1	3,652
1,048.11	8.2	3,678
1,048.12	8.2	3,704
1,048.13	8.3	3,730
1,048.14	8.3	3,757
1,048.15	8.4	3,783
1,048.16	8.5	3,809
1,048.17	8.5	3,836
1,048.18	8.6	3,862
1,048.19	8.6	3,889
1,048.20	8.7	3,916
1,048.21	8.8	3,943
1,048.22	8.8	3,969
1,048.23	8.9	3,996
1,048.24	8.9	4,023
1,048.25	9.0	4,050

SWM Calcs

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Type III 24-hr 100 YR Rainfall=7.29"

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Page 21

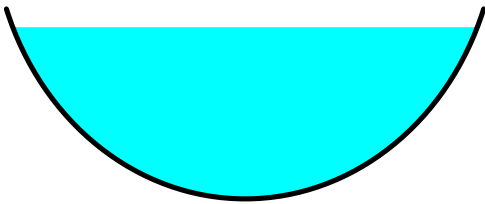
Summary for Reach 4R: Roadside swale

Inflow Area = 14.937 ac, 12.18% Impervious, Inflow Depth > 3.88" for 100 YR event
Inflow = 57.04 cfs @ 12.11 hrs, Volume= 4.825 af
Outflow = 55.09 cfs @ 12.15 hrs, Volume= 4.817 af, Atten= 3%, Lag= 2.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
Max. Velocity= 7.29 fps, Min. Travel Time= 1.1 min
Avg. Velocity= 3.02 fps, Avg. Travel Time= 2.5 min

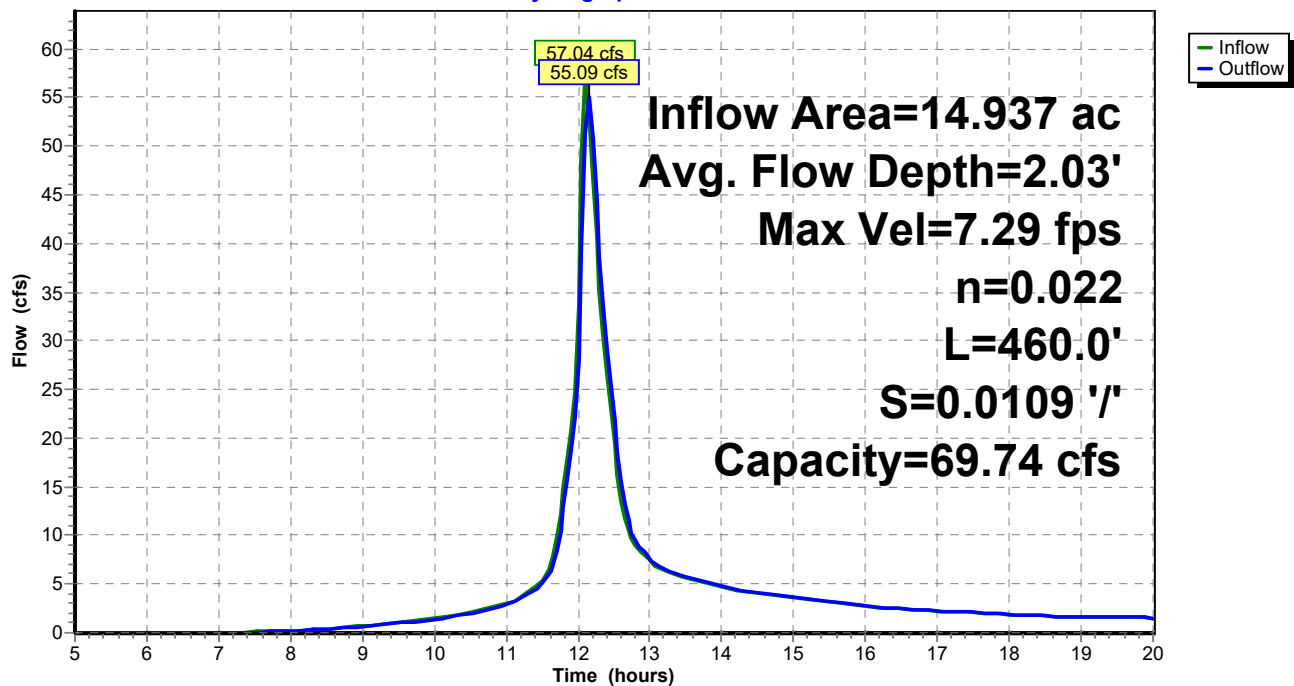
Peak Storage= 3,553 cf @ 12.12 hrs
Average Depth at Peak Storage= 2.03'
Bank-Full Depth= 2.25' Flow Area= 9.0 sf, Capacity= 69.74 cfs

6.00' x 2.25' deep Parabolic Channel, n= 0.022 Earth, clean & straight
Length= 460.0' Slope= 0.0109 '/'
Inlet Invert= 1,039.00', Outlet Invert= 1,034.00'



Reach 4R: Roadside swale

Hydrograph



SWM Calcs

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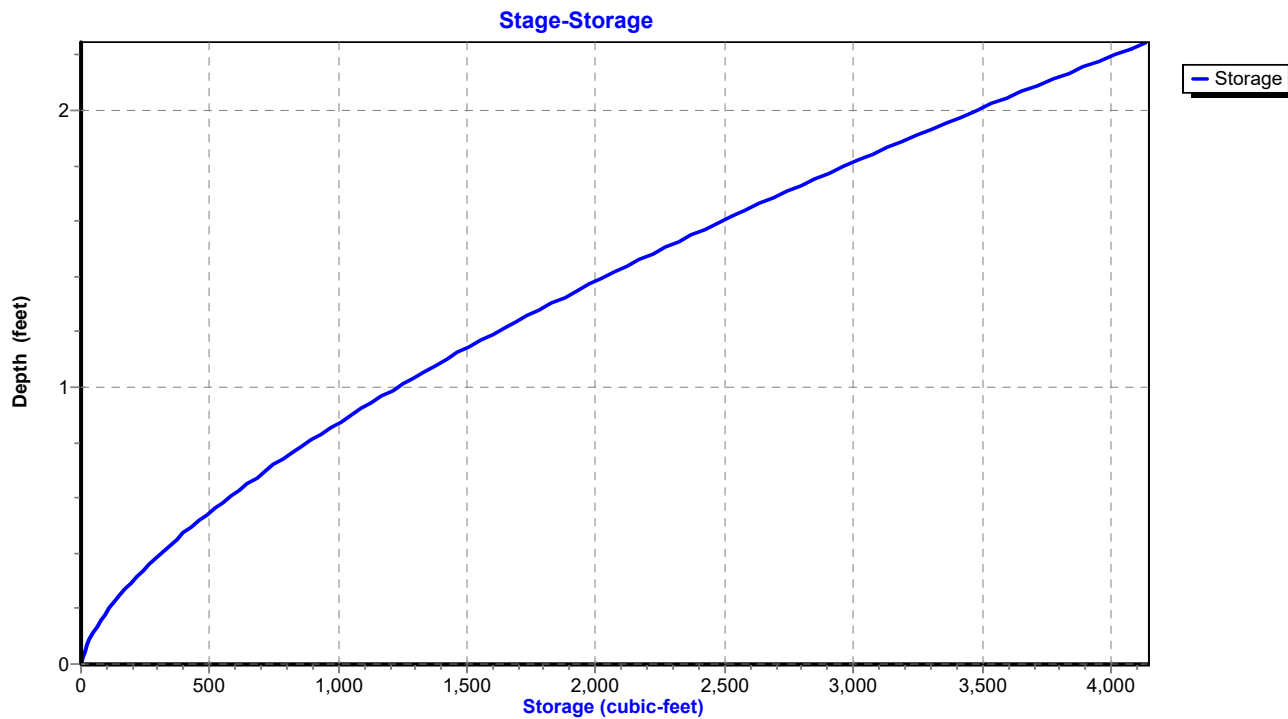
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Type III 24-hr 100 YR Rainfall=7.29"

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Page 22

Reach 4R: Roadside swale



SWM Calcs

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Type III 24-hr 100 YR Rainfall=7.29"

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Page 23

Stage-Area-Storage for Reach 4R: Roadside swale

Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)	Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)
1,039.00	0.0	0	1,039.52	1.0	460
1,039.01	0.0	2	1,039.53	1.0	473
1,039.02	0.0	4	1,039.54	1.1	487
1,039.03	0.0	7	1,039.55	1.1	500
1,039.04	0.0	10	1,039.56	1.1	514
1,039.05	0.0	14	1,039.57	1.1	528
1,039.06	0.0	18	1,039.58	1.2	542
1,039.07	0.0	23	1,039.59	1.2	556
1,039.08	0.1	28	1,039.60	1.2	570
1,039.09	0.1	33	1,039.61	1.3	584
1,039.10	0.1	39	1,039.62	1.3	599
1,039.11	0.1	45	1,039.63	1.3	613
1,039.12	0.1	51	1,039.64	1.4	628
1,039.13	0.1	58	1,039.65	1.4	643
1,039.14	0.1	64	1,039.66	1.4	658
1,039.15	0.2	71	1,039.67	1.5	673
1,039.16	0.2	79	1,039.68	1.5	688
1,039.17	0.2	86	1,039.69	1.5	703
1,039.18	0.2	94	1,039.70	1.6	718
1,039.19	0.2	102	1,039.71	1.6	734
1,039.20	0.2	110	1,039.72	1.6	749
1,039.21	0.3	118	1,039.73	1.7	765
1,039.22	0.3	127	1,039.74	1.7	781
1,039.23	0.3	135	1,039.75	1.7	797
1,039.24	0.3	144	1,039.76	1.8	813
1,039.25	0.3	153	1,039.77	1.8	829
1,039.26	0.4	163	1,039.78	1.8	845
1,039.27	0.4	172	1,039.79	1.9	861
1,039.28	0.4	182	1,039.80	1.9	878
1,039.29	0.4	192	1,039.81	1.9	894
1,039.30	0.4	202	1,039.82	2.0	911
1,039.31	0.5	212	1,039.83	2.0	928
1,039.32	0.5	222	1,039.84	2.1	944
1,039.33	0.5	233	1,039.85	2.1	961
1,039.34	0.5	243	1,039.86	2.1	978
1,039.35	0.6	254	1,039.87	2.2	995
1,039.36	0.6	265	1,039.88	2.2	1,013
1,039.37	0.6	276	1,039.89	2.2	1,030
1,039.38	0.6	287	1,039.90	2.3	1,047
1,039.39	0.6	299	1,039.91	2.3	1,065
1,039.40	0.7	310	1,039.92	2.4	1,082
1,039.41	0.7	322	1,039.93	2.4	1,100
1,039.42	0.7	334	1,039.94	2.4	1,118
1,039.43	0.8	346	1,039.95	2.5	1,136
1,039.44	0.8	358	1,039.96	2.5	1,154
1,039.45	0.8	370	1,039.97	2.5	1,172
1,039.46	0.8	383	1,039.98	2.6	1,190
1,039.47	0.9	395	1,039.99	2.6	1,208
1,039.48	0.9	408	1,040.00	2.7	1,227
1,039.49	0.9	421	1,040.01	2.7	1,245
1,039.50	0.9	434	1,040.02	2.7	1,264
1,039.51	1.0	447	1,040.03	2.8	1,282

SWM Calcs

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Type III 24-hr 100 YR Rainfall=7.29"

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Page 24

Stage-Area-Storage for Reach 4R: Roadside swale (continued)

Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)	Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)
1,040.04	2.8	1,301	1,040.56	5.2	2,390
1,040.05	2.9	1,320	1,040.57	5.2	2,413
1,040.06	2.9	1,339	1,040.58	5.3	2,436
1,040.07	3.0	1,358	1,040.59	5.3	2,459
1,040.08	3.0	1,377	1,040.60	5.4	2,483
1,040.09	3.0	1,396	1,040.61	5.4	2,506
1,040.10	3.1	1,415	1,040.62	5.5	2,529
1,040.11	3.1	1,435	1,040.63	5.5	2,553
1,040.12	3.2	1,454	1,040.64	5.6	2,576
1,040.13	3.2	1,474	1,040.65	5.7	2,600
1,040.14	3.2	1,493	1,040.66	5.7	2,624
1,040.15	3.3	1,513	1,040.67	5.8	2,647
1,040.16	3.3	1,533	1,040.68	5.8	2,671
1,040.17	3.4	1,552	1,040.69	5.9	2,695
1,040.18	3.4	1,572	1,040.70	5.9	2,719
1,040.19	3.5	1,592	1,040.71	6.0	2,743
1,040.20	3.5	1,613	1,040.72	6.0	2,767
1,040.21	3.5	1,633	1,040.73	6.1	2,791
1,040.22	3.6	1,653	1,040.74	6.1	2,816
1,040.23	3.6	1,673	1,040.75	6.2	2,840
1,040.24	3.7	1,694	1,040.76	6.2	2,864
1,040.25	3.7	1,714	1,040.77	6.3	2,889
1,040.26	3.8	1,735	1,040.78	6.3	2,913
1,040.27	3.8	1,756	1,040.79	6.4	2,938
1,040.28	3.9	1,776	1,040.80	6.4	2,962
1,040.29	3.9	1,797	1,040.81	6.5	2,987
1,040.30	4.0	1,818	1,040.82	6.5	3,012
1,040.31	4.0	1,839	1,040.83	6.6	3,037
1,040.32	4.0	1,860	1,040.84	6.7	3,062
1,040.33	4.1	1,882	1,040.85	6.7	3,087
1,040.34	4.1	1,903	1,040.86	6.8	3,112
1,040.35	4.2	1,924	1,040.87	6.8	3,137
1,040.36	4.2	1,946	1,040.88	6.9	3,162
1,040.37	4.3	1,967	1,040.89	6.9	3,187
1,040.38	4.3	1,989	1,040.90	7.0	3,213
1,040.39	4.4	2,010	1,040.91	7.0	3,238
1,040.40	4.4	2,032	1,040.92	7.1	3,263
1,040.41	4.5	2,054	1,040.93	7.2	3,289
1,040.42	4.5	2,076	1,040.94	7.2	3,315
1,040.43	4.6	2,098	1,040.95	7.3	3,340
1,040.44	4.6	2,120	1,040.96	7.3	3,366
1,040.45	4.7	2,142	1,040.97	7.4	3,392
1,040.46	4.7	2,164	1,040.98	7.4	3,418
1,040.47	4.8	2,186	1,040.99	7.5	3,444
1,040.48	4.8	2,209	1,041.00	7.5	3,470
1,040.49	4.9	2,231	1,041.01	7.6	3,496
1,040.50	4.9	2,254	1,041.02	7.7	3,522
1,040.51	4.9	2,276	1,041.03	7.7	3,548
1,040.52	5.0	2,299	1,041.04	7.8	3,574
1,040.53	5.0	2,321	1,041.05	7.8	3,600
1,040.54	5.1	2,344	1,041.06	7.9	3,627
1,040.55	5.1	2,367	1,041.07	7.9	3,653

SWM Calcs

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Type III 24-hr 100 YR Rainfall=7.29"

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Page 25

Stage-Area-Storage for Reach 4R: Roadside swale (continued)

Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)
1,041.08	8.0	3,680
1,041.09	8.1	3,706
1,041.10	8.1	3,733
1,041.11	8.2	3,760
1,041.12	8.2	3,786
1,041.13	8.3	3,813
1,041.14	8.3	3,840
1,041.15	8.4	3,867
1,041.16	8.5	3,894
1,041.17	8.5	3,921
1,041.18	8.6	3,948
1,041.19	8.6	3,976
1,041.20	8.7	4,003
1,041.21	8.8	4,030
1,041.22	8.8	4,058
1,041.23	8.9	4,085
1,041.24	8.9	4,112
1,041.25	9.0	4,140

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Page 26

Summary for Reach 10R: Culvert at Curb Cut

Inflow Area = 5.010 ac, 7.30% Impervious, Inflow Depth > 4.11" for 100 YR event
Inflow = 21.01 cfs @ 12.20 hrs, Volume= 1.716 af
Outflow = 20.92 cfs @ 12.20 hrs, Volume= 1.715 af, Atten= 0%, Lag= 0.3 min

Routing by Stor-Ind+Trans method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Max. Velocity= 8.24 fps, Min. Travel Time= 0.2 min

Avg. Velocity = 2.75 fps, Avg. Travel Time= 0.6 min

Peak Storage= 255 cf @ 12.20 hrs

Average Depth at Peak Storage= 0.85'

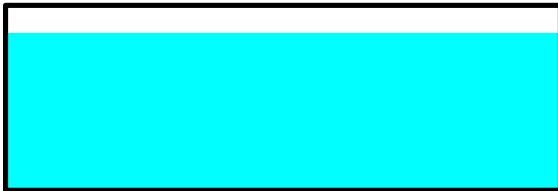
Bank-Full Depth= 1.00' Flow Area= 3.0 sf, Capacity= 19.32 cfs

36.0" W x 12.0" H Box Pipe

n= 0.012

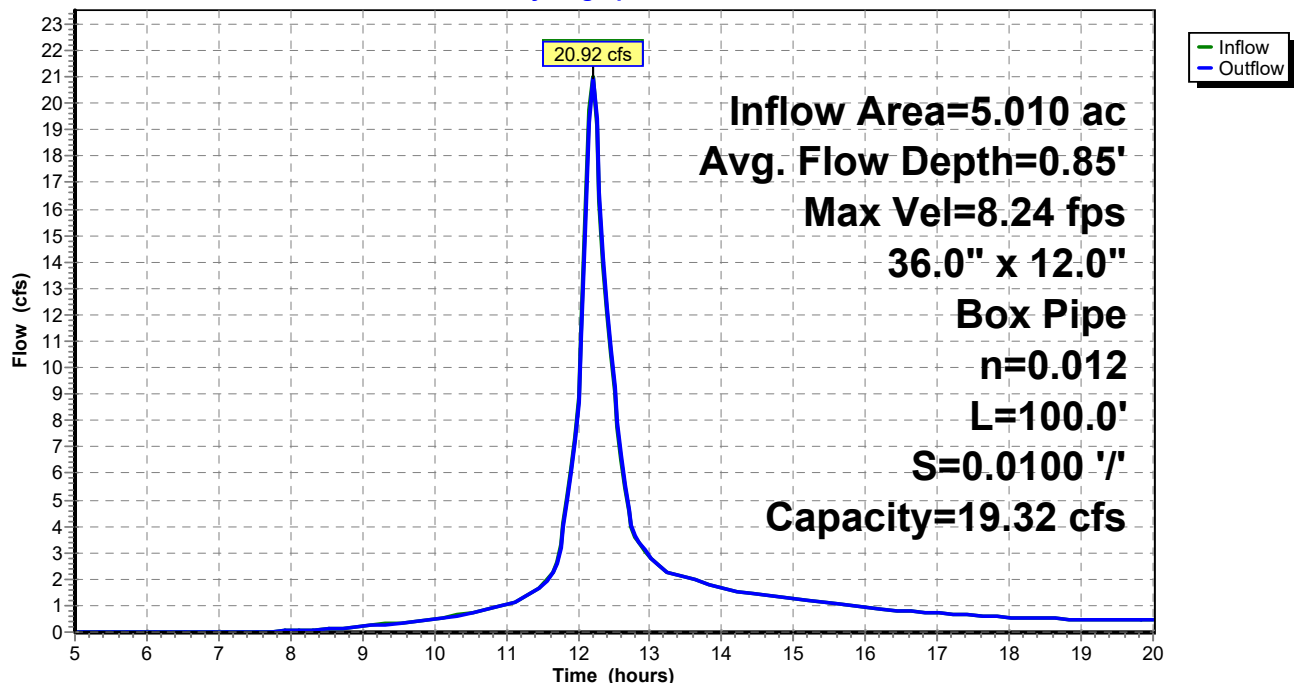
Length= 100.0' Slope= 0.0100 '/'

Inlet Invert= 1,040.00', Outlet Invert= 1,039.00'



Reach 10R: Culvert at Curb Cut

Hydrograph



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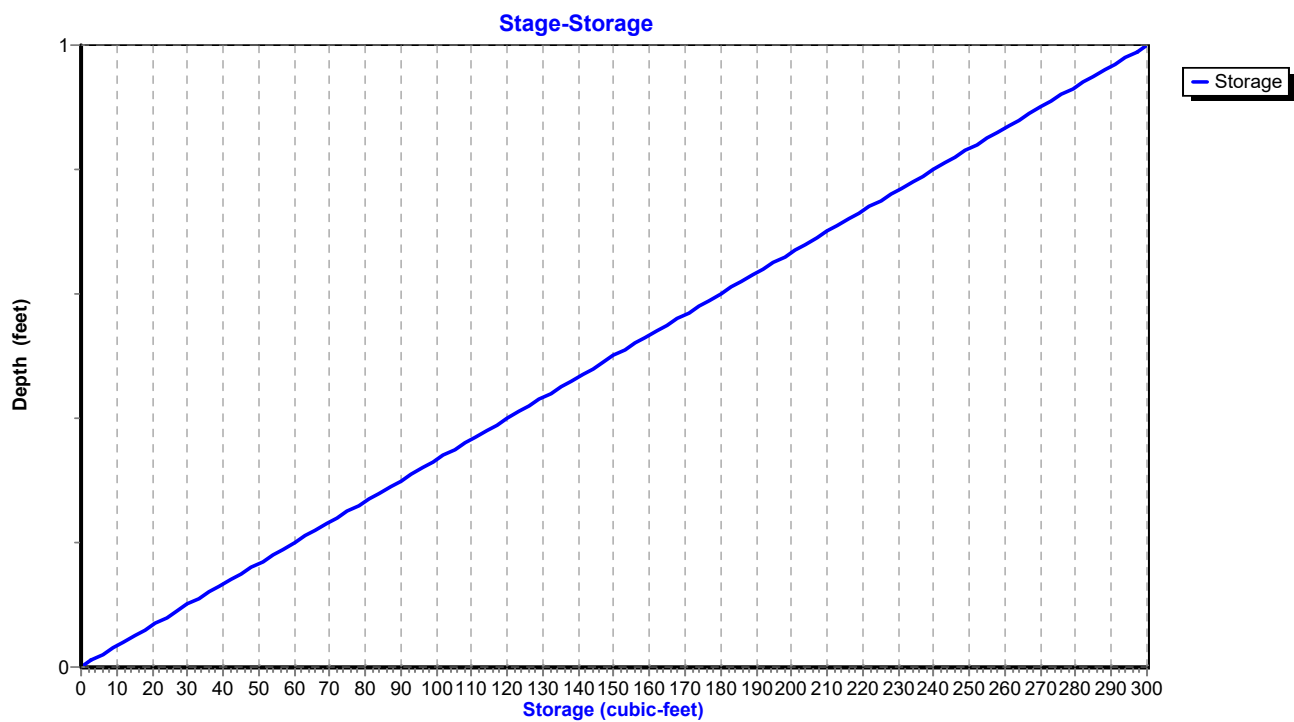
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Type III 24-hr 100 YR Rainfall=7.29"

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Page 27

Reach 10R: Culvert at Curb Cut



SWM Calcs

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Page 28

Stage-Area-Storage for Reach 10R: Culvert at Curb Cut

Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)	Elevation (feet)	End-Area (sq-ft)	Storage (cubic-feet)
1,040.00	0.0	0	1,040.52	1.6	156
1,040.01	0.0	3	1,040.53	1.6	159
1,040.02	0.1	6	1,040.54	1.6	162
1,040.03	0.1	9	1,040.55	1.7	165
1,040.04	0.1	12	1,040.56	1.7	168
1,040.05	0.2	15	1,040.57	1.7	171
1,040.06	0.2	18	1,040.58	1.7	174
1,040.07	0.2	21	1,040.59	1.8	177
1,040.08	0.2	24	1,040.60	1.8	180
1,040.09	0.3	27	1,040.61	1.8	183
1,040.10	0.3	30	1,040.62	1.9	186
1,040.11	0.3	33	1,040.63	1.9	189
1,040.12	0.4	36	1,040.64	1.9	192
1,040.13	0.4	39	1,040.65	2.0	195
1,040.14	0.4	42	1,040.66	2.0	198
1,040.15	0.4	45	1,040.67	2.0	201
1,040.16	0.5	48	1,040.68	2.0	204
1,040.17	0.5	51	1,040.69	2.1	207
1,040.18	0.5	54	1,040.70	2.1	210
1,040.19	0.6	57	1,040.71	2.1	213
1,040.20	0.6	60	1,040.72	2.2	216
1,040.21	0.6	63	1,040.73	2.2	219
1,040.22	0.7	66	1,040.74	2.2	222
1,040.23	0.7	69	1,040.75	2.3	225
1,040.24	0.7	72	1,040.76	2.3	228
1,040.25	0.8	75	1,040.77	2.3	231
1,040.26	0.8	78	1,040.78	2.3	234
1,040.27	0.8	81	1,040.79	2.4	237
1,040.28	0.8	84	1,040.80	2.4	240
1,040.29	0.9	87	1,040.81	2.4	243
1,040.30	0.9	90	1,040.82	2.5	246
1,040.31	0.9	93	1,040.83	2.5	249
1,040.32	1.0	96	1,040.84	2.5	252
1,040.33	1.0	99	1,040.85	2.5	255
1,040.34	1.0	102	1,040.86	2.6	258
1,040.35	1.1	105	1,040.87	2.6	261
1,040.36	1.1	108	1,040.88	2.6	264
1,040.37	1.1	111	1,040.89	2.7	267
1,040.38	1.1	114	1,040.90	2.7	270
1,040.39	1.2	117	1,040.91	2.7	273
1,040.40	1.2	120	1,040.92	2.8	276
1,040.41	1.2	123	1,040.93	2.8	279
1,040.42	1.3	126	1,040.94	2.8	282
1,040.43	1.3	129	1,040.95	2.9	285
1,040.44	1.3	132	1,040.96	2.9	288
1,040.45	1.4	135	1,040.97	2.9	291
1,040.46	1.4	138	1,040.98	2.9	294
1,040.47	1.4	141	1,040.99	3.0	297
1,040.48	1.4	144	1,041.00	3.0	300
1,040.49	1.5	147			
1,040.50	1.5	150			
1,040.51	1.5	153			

SWM Calcs

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Type III 24-hr 100 YR Rainfall=7.29"

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Page 29

Summary for Pond 5P: (new Pond)

Inflow Area = 1.871 ac, 29.67% Impervious, Inflow Depth > 4.67" for 100 YR event
 Inflow = 11.47 cfs @ 12.06 hrs, Volume= 0.728 af
 Outflow = 0.43 cfs @ 15.33 hrs, Volume= 0.270 af, Atten= 96%, Lag= 196.2 min
 Primary = 0.43 cfs @ 15.33 hrs, Volume= 0.270 af

Routing by Stor-Ind method, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs
 Peak Elev= 1,041.87' @ 15.33 hrs Surf.Area= 7,561 sf Storage= 22,741 cf

Plug-Flow detention time= 287.3 min calculated for 0.270 af (37% of inflow)
 Center-of-Mass det. time= 191.4 min (963.7 - 772.2)

Volume	Invert	Avail.Storage	Storage Description
#1	1,038.00'	31,830 cf	Custom Stage Data (Prismatic) Listed below (Recalc) x 2

Elevation (feet)	Surf.Area (sq-ft)	Inc.Store (cubic-feet)	Cum.Store (cubic-feet)
1,038.00	2,094	0	0
1,043.00	4,272	15,915	15,915

Device	Routing	Invert	Outlet Devices
#1	Primary	1,042.08'	5.0' long x 5.0' breadth Broad-Crested Rectangular Weir Head (feet) 0.20 0.40 0.60 0.80 1.00 1.20 1.40 1.60 1.80 2.00 2.50 3.00 3.50 4.00 4.50 5.00 5.50 Coef. (English) 2.34 2.50 2.70 2.68 2.68 2.66 2.65 2.65 2.65 2.65 2.67 2.66 2.68 2.70 2.74 2.79 2.88
#2	Primary	1,040.00'	4.0" Round Culvert L= 32.0' CPP, mitered to conform to fill, Ke= 0.700 Inlet / Outlet Invert= 1,040.00' / 1,039.50' S= 0.0156 '/' Cc= 0.900 n= 0.012, Flow Area= 0.09 sf

Primary OutFlow Max=0.43 cfs @ 15.33 hrs HW=1,041.87' (Free Discharge)

1=Broad-Crested Rectangular Weir (Controls 0.00 cfs)

2=Culvert (Barrel Controls 0.43 cfs @ 4.93 fps)

SWM Calcs

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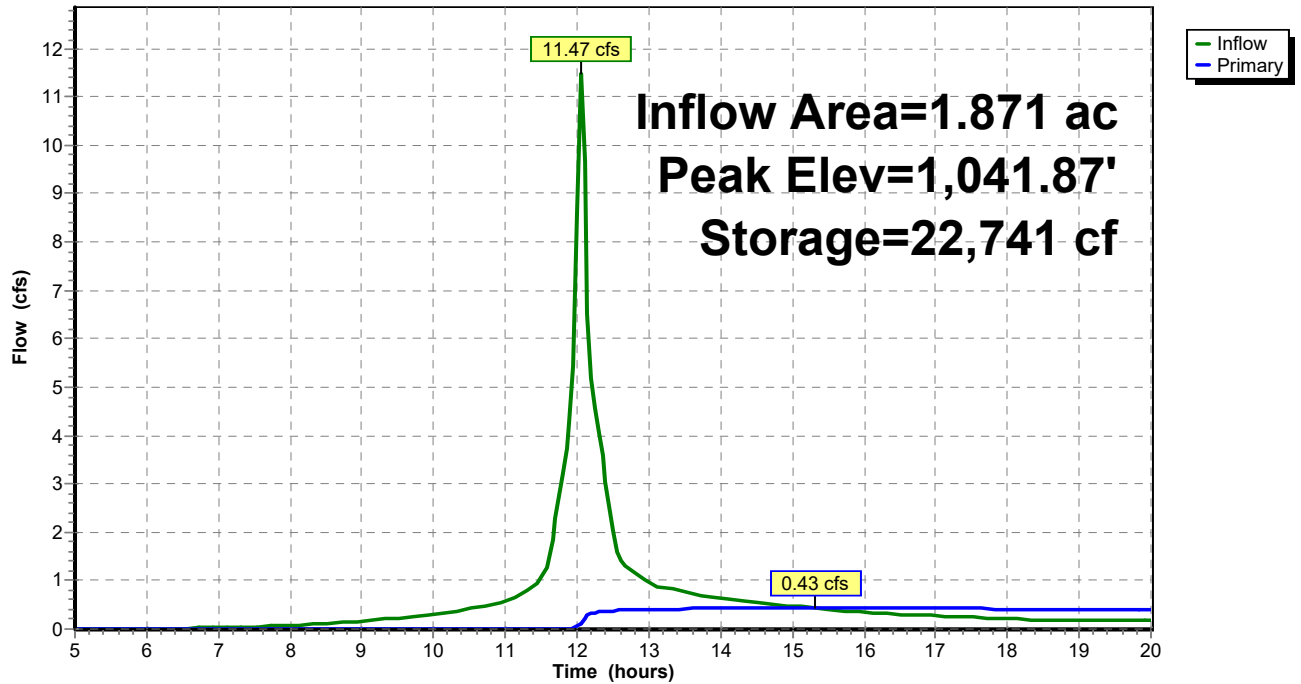
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Type III 24-hr 100 YR Rainfall=7.29"

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Page 30

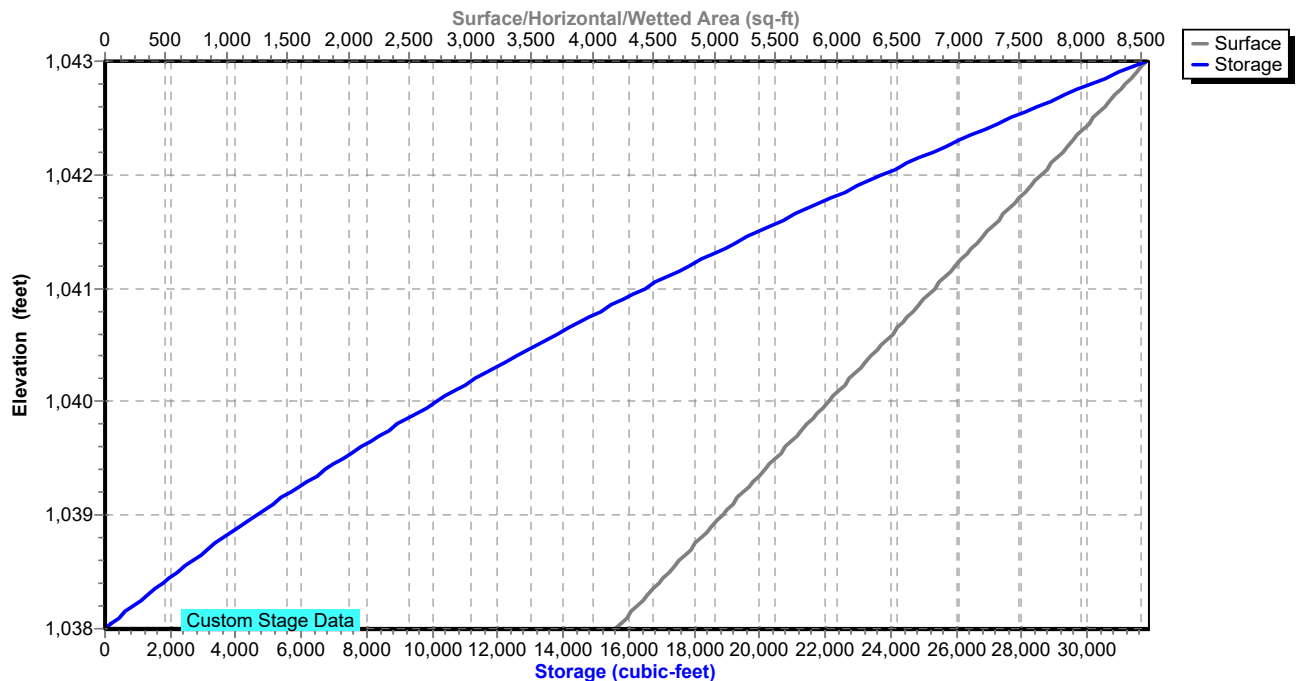
Pond 5P: (new Pond)

Hydrograph



Pond 5P: (new Pond)

Stage-Area-Storage



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Type III 24-hr 100 YR Rainfall=7.29"

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Page 31

Stage-Area-Storage for Pond 5P: (new Pond)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
1,038.00	4,188	0	1,038.52	4,641	2,296
1,038.01	4,197	42	1,038.53	4,650	2,342
1,038.02	4,205	84	1,038.54	4,658	2,389
1,038.03	4,214	126	1,038.55	4,667	2,435
1,038.04	4,223	168	1,038.56	4,676	2,482
1,038.05	4,232	210	1,038.57	4,685	2,529
1,038.06	4,240	253	1,038.58	4,693	2,576
1,038.07	4,249	295	1,038.59	4,702	2,623
1,038.08	4,258	338	1,038.60	4,711	2,670
1,038.09	4,266	380	1,038.61	4,719	2,717
1,038.10	4,275	423	1,038.62	4,728	2,764
1,038.11	4,284	466	1,038.63	4,737	2,811
1,038.12	4,293	509	1,038.64	4,746	2,859
1,038.13	4,301	552	1,038.65	4,754	2,906
1,038.14	4,310	595	1,038.66	4,763	2,954
1,038.15	4,319	638	1,038.67	4,772	3,002
1,038.16	4,327	681	1,038.68	4,780	3,049
1,038.17	4,336	725	1,038.69	4,789	3,097
1,038.18	4,345	768	1,038.70	4,798	3,145
1,038.19	4,354	811	1,038.71	4,807	3,193
1,038.20	4,362	855	1,038.72	4,815	3,241
1,038.21	4,371	899	1,038.73	4,824	3,289
1,038.22	4,380	942	1,038.74	4,833	3,338
1,038.23	4,388	986	1,038.75	4,841	3,386
1,038.24	4,397	1,030	1,038.76	4,850	3,434
1,038.25	4,406	1,074	1,038.77	4,859	3,483
1,038.26	4,415	1,118	1,038.78	4,868	3,532
1,038.27	4,423	1,163	1,038.79	4,876	3,580
1,038.28	4,432	1,207	1,038.80	4,885	3,629
1,038.29	4,441	1,251	1,038.81	4,894	3,678
1,038.30	4,449	1,296	1,038.82	4,902	3,727
1,038.31	4,458	1,340	1,038.83	4,911	3,776
1,038.32	4,467	1,385	1,038.84	4,920	3,825
1,038.33	4,475	1,429	1,038.85	4,929	3,875
1,038.34	4,484	1,474	1,038.86	4,937	3,924
1,038.35	4,493	1,519	1,038.87	4,946	3,973
1,038.36	4,502	1,564	1,038.88	4,955	4,023
1,038.37	4,510	1,609	1,038.89	4,963	4,072
1,038.38	4,519	1,654	1,038.90	4,972	4,122
1,038.39	4,528	1,700	1,038.91	4,981	4,172
1,038.40	4,536	1,745	1,038.92	4,990	4,222
1,038.41	4,545	1,790	1,038.93	4,998	4,272
1,038.42	4,554	1,836	1,038.94	5,007	4,322
1,038.43	4,563	1,881	1,038.95	5,016	4,372
1,038.44	4,571	1,927	1,038.96	5,024	4,422
1,038.45	4,580	1,973	1,038.97	5,033	4,472
1,038.46	4,589	2,019	1,038.98	5,042	4,523
1,038.47	4,597	2,065	1,038.99	5,050	4,573
1,038.48	4,606	2,111	1,039.00	5,059	4,624
1,038.49	4,615	2,157	1,039.01	5,068	4,674
1,038.50	4,624	2,203	1,039.02	5,077	4,725
1,038.51	4,632	2,249	1,039.03	5,085	4,776

SWM Calcs

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Type III 24-hr 100 YR Rainfall=7.29"

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Page 32

Stage-Area-Storage for Pond 5P: (new Pond) (continued)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
1,039.04	5,094	4,827	1,039.56	5,547	7,593
1,039.05	5,103	4,878	1,039.57	5,556	7,649
1,039.06	5,111	4,929	1,039.58	5,564	7,704
1,039.07	5,120	4,980	1,039.59	5,573	7,760
1,039.08	5,129	5,031	1,039.60	5,582	7,816
1,039.09	5,138	5,082	1,039.61	5,591	7,872
1,039.10	5,146	5,134	1,039.62	5,599	7,928
1,039.11	5,155	5,185	1,039.63	5,608	7,984
1,039.12	5,164	5,237	1,039.64	5,617	8,040
1,039.13	5,172	5,289	1,039.65	5,625	8,096
1,039.14	5,181	5,340	1,039.66	5,634	8,152
1,039.15	5,190	5,392	1,039.67	5,643	8,209
1,039.16	5,199	5,444	1,039.68	5,652	8,265
1,039.17	5,207	5,496	1,039.69	5,660	8,322
1,039.18	5,216	5,548	1,039.70	5,669	8,378
1,039.19	5,225	5,601	1,039.71	5,678	8,435
1,039.20	5,233	5,653	1,039.72	5,686	8,492
1,039.21	5,242	5,705	1,039.73	5,695	8,549
1,039.22	5,251	5,758	1,039.74	5,704	8,606
1,039.23	5,260	5,810	1,039.75	5,713	8,663
1,039.24	5,268	5,863	1,039.76	5,721	8,720
1,039.25	5,277	5,916	1,039.77	5,730	8,777
1,039.26	5,286	5,968	1,039.78	5,739	8,835
1,039.27	5,294	6,021	1,039.79	5,747	8,892
1,039.28	5,303	6,074	1,039.80	5,756	8,950
1,039.29	5,312	6,127	1,039.81	5,765	9,007
1,039.30	5,321	6,181	1,039.82	5,774	9,065
1,039.31	5,329	6,234	1,039.83	5,782	9,123
1,039.32	5,338	6,287	1,039.84	5,791	9,181
1,039.33	5,347	6,341	1,039.85	5,800	9,239
1,039.34	5,355	6,394	1,039.86	5,808	9,297
1,039.35	5,364	6,448	1,039.87	5,817	9,355
1,039.36	5,373	6,501	1,039.88	5,826	9,413
1,039.37	5,382	6,555	1,039.89	5,835	9,471
1,039.38	5,390	6,609	1,039.90	5,843	9,530
1,039.39	5,399	6,663	1,039.91	5,852	9,588
1,039.40	5,408	6,717	1,039.92	5,861	9,647
1,039.41	5,416	6,771	1,039.93	5,869	9,705
1,039.42	5,425	6,825	1,039.94	5,878	9,764
1,039.43	5,434	6,880	1,039.95	5,887	9,823
1,039.44	5,443	6,934	1,039.96	5,896	9,882
1,039.45	5,451	6,988	1,039.97	5,904	9,941
1,039.46	5,460	7,043	1,039.98	5,913	10,000
1,039.47	5,469	7,098	1,039.99	5,922	10,059
1,039.48	5,477	7,152	1,040.00	5,930	10,118
1,039.49	5,486	7,207	1,040.01	5,939	10,178
1,039.50	5,495	7,262	1,040.02	5,948	10,237
1,039.51	5,504	7,317	1,040.03	5,957	10,297
1,039.52	5,512	7,372	1,040.04	5,965	10,356
1,039.53	5,521	7,427	1,040.05	5,974	10,416
1,039.54	5,530	7,483	1,040.06	5,983	10,476
1,039.55	5,538	7,538	1,040.07	5,991	10,536

SWM Calcs

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Type III 24-hr 100 YR Rainfall=7.29"

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Page 33

Stage-Area-Storage for Pond 5P: (new Pond) (continued)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
1,040.08	6,000	10,596	1,040.60	6,453	13,833
1,040.09	6,009	10,656	1,040.61	6,462	13,898
1,040.10	6,018	10,716	1,040.62	6,471	13,963
1,040.11	6,026	10,776	1,040.63	6,479	14,027
1,040.12	6,035	10,836	1,040.64	6,488	14,092
1,040.13	6,044	10,897	1,040.65	6,497	14,157
1,040.14	6,052	10,957	1,040.66	6,505	14,222
1,040.15	6,061	11,018	1,040.67	6,514	14,287
1,040.16	6,070	11,078	1,040.68	6,523	14,352
1,040.17	6,079	11,139	1,040.69	6,532	14,418
1,040.18	6,087	11,200	1,040.70	6,540	14,483
1,040.19	6,096	11,261	1,040.71	6,549	14,549
1,040.20	6,105	11,322	1,040.72	6,558	14,614
1,040.21	6,113	11,383	1,040.73	6,566	14,680
1,040.22	6,122	11,444	1,040.74	6,575	14,745
1,040.23	6,131	11,505	1,040.75	6,584	14,811
1,040.24	6,139	11,567	1,040.76	6,593	14,877
1,040.25	6,148	11,628	1,040.77	6,601	14,943
1,040.26	6,157	11,690	1,040.78	6,610	15,009
1,040.27	6,166	11,751	1,040.79	6,619	15,075
1,040.28	6,174	11,813	1,040.80	6,627	15,142
1,040.29	6,183	11,875	1,040.81	6,636	15,208
1,040.30	6,192	11,937	1,040.82	6,645	15,274
1,040.31	6,200	11,999	1,040.83	6,653	15,341
1,040.32	6,209	12,061	1,040.84	6,662	15,407
1,040.33	6,218	12,123	1,040.85	6,671	15,474
1,040.34	6,227	12,185	1,040.86	6,680	15,541
1,040.35	6,235	12,247	1,040.87	6,688	15,608
1,040.36	6,244	12,310	1,040.88	6,697	15,674
1,040.37	6,253	12,372	1,040.89	6,706	15,741
1,040.38	6,261	12,435	1,040.90	6,714	15,809
1,040.39	6,270	12,498	1,040.91	6,723	15,876
1,040.40	6,279	12,560	1,040.92	6,732	15,943
1,040.41	6,288	12,623	1,040.93	6,741	16,010
1,040.42	6,296	12,686	1,040.94	6,749	16,078
1,040.43	6,305	12,749	1,040.95	6,758	16,145
1,040.44	6,314	12,812	1,040.96	6,767	16,213
1,040.45	6,322	12,875	1,040.97	6,775	16,281
1,040.46	6,331	12,939	1,040.98	6,784	16,349
1,040.47	6,340	13,002	1,040.99	6,793	16,416
1,040.48	6,349	13,065	1,041.00	6,802	16,484
1,040.49	6,357	13,129	1,041.01	6,810	16,552
1,040.50	6,366	13,193	1,041.02	6,819	16,621
1,040.51	6,375	13,256	1,041.03	6,828	16,689
1,040.52	6,383	13,320	1,041.04	6,836	16,757
1,040.53	6,392	13,384	1,041.05	6,845	16,826
1,040.54	6,401	13,448	1,041.06	6,854	16,894
1,040.55	6,410	13,512	1,041.07	6,863	16,963
1,040.56	6,418	13,576	1,041.08	6,871	17,031
1,040.57	6,427	13,640	1,041.09	6,880	17,100
1,040.58	6,436	13,705	1,041.10	6,889	17,169
1,040.59	6,444	13,769	1,041.11	6,897	17,238

SWM Calcs

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Type III 24-hr 100 YR Rainfall=7.29"

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Page 34

Stage-Area-Storage for Pond 5P: (new Pond) (continued)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
1,041.12	6,906	17,307	1,041.64	7,359	21,016
1,041.13	6,915	17,376	1,041.65	7,368	21,089
1,041.14	6,924	17,445	1,041.66	7,377	21,163
1,041.15	6,932	17,514	1,041.67	7,385	21,237
1,041.16	6,941	17,584	1,041.68	7,394	21,311
1,041.17	6,950	17,653	1,041.69	7,403	21,385
1,041.18	6,958	17,723	1,041.70	7,411	21,459
1,041.19	6,967	17,792	1,041.71	7,420	21,533
1,041.20	6,976	17,862	1,041.72	7,429	21,607
1,041.21	6,985	17,932	1,041.73	7,438	21,682
1,041.22	6,993	18,002	1,041.74	7,446	21,756
1,041.23	7,002	18,072	1,041.75	7,455	21,831
1,041.24	7,011	18,142	1,041.76	7,464	21,905
1,041.25	7,019	18,212	1,041.77	7,472	21,980
1,041.26	7,028	18,282	1,041.78	7,481	22,055
1,041.27	7,037	18,353	1,041.79	7,490	22,130
1,041.28	7,046	18,423	1,041.80	7,499	22,204
1,041.29	7,054	18,493	1,041.81	7,507	22,279
1,041.30	7,063	18,564	1,041.82	7,516	22,355
1,041.31	7,072	18,635	1,041.83	7,525	22,430
1,041.32	7,080	18,706	1,041.84	7,533	22,505
1,041.33	7,089	18,776	1,041.85	7,542	22,580
1,041.34	7,098	18,847	1,041.86	7,551	22,656
1,041.35	7,107	18,918	1,041.87	7,560	22,731
1,041.36	7,115	18,989	1,041.88	7,568	22,807
1,041.37	7,124	19,061	1,041.89	7,577	22,883
1,041.38	7,133	19,132	1,041.90	7,586	22,959
1,041.39	7,141	19,203	1,041.91	7,594	23,035
1,041.40	7,150	19,275	1,041.92	7,603	23,111
1,041.41	7,159	19,346	1,041.93	7,612	23,187
1,041.42	7,168	19,418	1,041.94	7,621	23,263
1,041.43	7,176	19,490	1,041.95	7,629	23,339
1,041.44	7,185	19,561	1,041.96	7,638	23,415
1,041.45	7,194	19,633	1,041.97	7,647	23,492
1,041.46	7,202	19,705	1,041.98	7,655	23,568
1,041.47	7,211	19,777	1,041.99	7,664	23,645
1,041.48	7,220	19,850	1,042.00	7,673	23,722
1,041.49	7,228	19,922	1,042.01	7,682	23,798
1,041.50	7,237	19,994	1,042.02	7,690	23,875
1,041.51	7,246	20,067	1,042.03	7,699	23,952
1,041.52	7,255	20,139	1,042.04	7,708	24,029
1,041.53	7,263	20,212	1,042.05	7,716	24,106
1,041.54	7,272	20,284	1,042.06	7,725	24,184
1,041.55	7,281	20,357	1,042.07	7,734	24,261
1,041.56	7,289	20,430	1,042.08	7,742	24,338
1,041.57	7,298	20,503	1,042.09	7,751	24,416
1,041.58	7,307	20,576	1,042.10	7,760	24,493
1,041.59	7,316	20,649	1,042.11	7,769	24,571
1,041.60	7,324	20,722	1,042.12	7,777	24,649
1,041.61	7,333	20,795	1,042.13	7,786	24,726
1,041.62	7,342	20,869	1,042.14	7,795	24,804
1,041.63	7,350	20,942	1,042.15	7,803	24,882

SWM Calcs

Prepared by SK Design Group, Inc

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Stockbridge Subdivision

Type III 24-hr 100 YR Rainfall=7.29"

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Page 35

Stage-Area-Storage for Pond 5P: (new Pond) (continued)

Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)	Elevation (feet)	Surface (sq-ft)	Storage (cubic-feet)
1,042.16	7,812	24,960	1,042.68	8,265	29,141
1,042.17	7,821	25,039	1,042.69	8,274	29,223
1,042.18	7,830	25,117	1,042.70	8,283	29,306
1,042.19	7,838	25,195	1,042.71	8,291	29,389
1,042.20	7,847	25,274	1,042.72	8,300	29,472
1,042.21	7,856	25,352	1,042.73	8,309	29,555
1,042.22	7,864	25,431	1,042.74	8,317	29,638
1,042.23	7,873	25,509	1,042.75	8,326	29,721
1,042.24	7,882	25,588	1,042.76	8,335	29,805
1,042.25	7,891	25,667	1,042.77	8,344	29,888
1,042.26	7,899	25,746	1,042.78	8,352	29,971
1,042.27	7,908	25,825	1,042.79	8,361	30,055
1,042.28	7,917	25,904	1,042.80	8,370	30,139
1,042.29	7,925	25,983	1,042.81	8,378	30,222
1,042.30	7,934	26,063	1,042.82	8,387	30,306
1,042.31	7,943	26,142	1,042.83	8,396	30,390
1,042.32	7,952	26,222	1,042.84	8,405	30,474
1,042.33	7,960	26,301	1,042.85	8,413	30,558
1,042.34	7,969	26,381	1,042.86	8,422	30,642
1,042.35	7,978	26,460	1,042.87	8,431	30,727
1,042.36	7,986	26,540	1,042.88	8,439	30,811
1,042.37	7,995	26,620	1,042.89	8,448	30,895
1,042.38	8,004	26,700	1,042.90	8,457	30,980
1,042.39	8,013	26,780	1,042.91	8,466	31,065
1,042.40	8,021	26,860	1,042.92	8,474	31,149
1,042.41	8,030	26,941	1,042.93	8,483	31,234
1,042.42	8,039	27,021	1,042.94	8,492	31,319
1,042.43	8,047	27,101	1,042.95	8,500	31,404
1,042.44	8,056	27,182	1,042.96	8,509	31,489
1,042.45	8,065	27,263	1,042.97	8,518	31,574
1,042.46	8,074	27,343	1,042.98	8,527	31,659
1,042.47	8,082	27,424	1,042.99	8,535	31,745
1,042.48	8,091	27,505	1,043.00	8,544	31,830
1,042.49	8,100	27,586			
1,042.50	8,108	27,667			
1,042.51	8,117	27,748			
1,042.52	8,126	27,829			
1,042.53	8,135	27,911			
1,042.54	8,143	27,992			
1,042.55	8,152	28,073			
1,042.56	8,161	28,155			
1,042.57	8,169	28,237			
1,042.58	8,178	28,318			
1,042.59	8,187	28,400			
1,042.60	8,196	28,482			
1,042.61	8,204	28,564			
1,042.62	8,213	28,646			
1,042.63	8,222	28,728			
1,042.64	8,230	28,811			
1,042.65	8,239	28,893			
1,042.66	8,248	28,975			
1,042.67	8,257	29,058			

SWM Calcs

Prepared by SK Design Group, Inc

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Stockbridge Subdivision

Type III 24-hr 100 YR Rainfall=7.29"

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Page 36

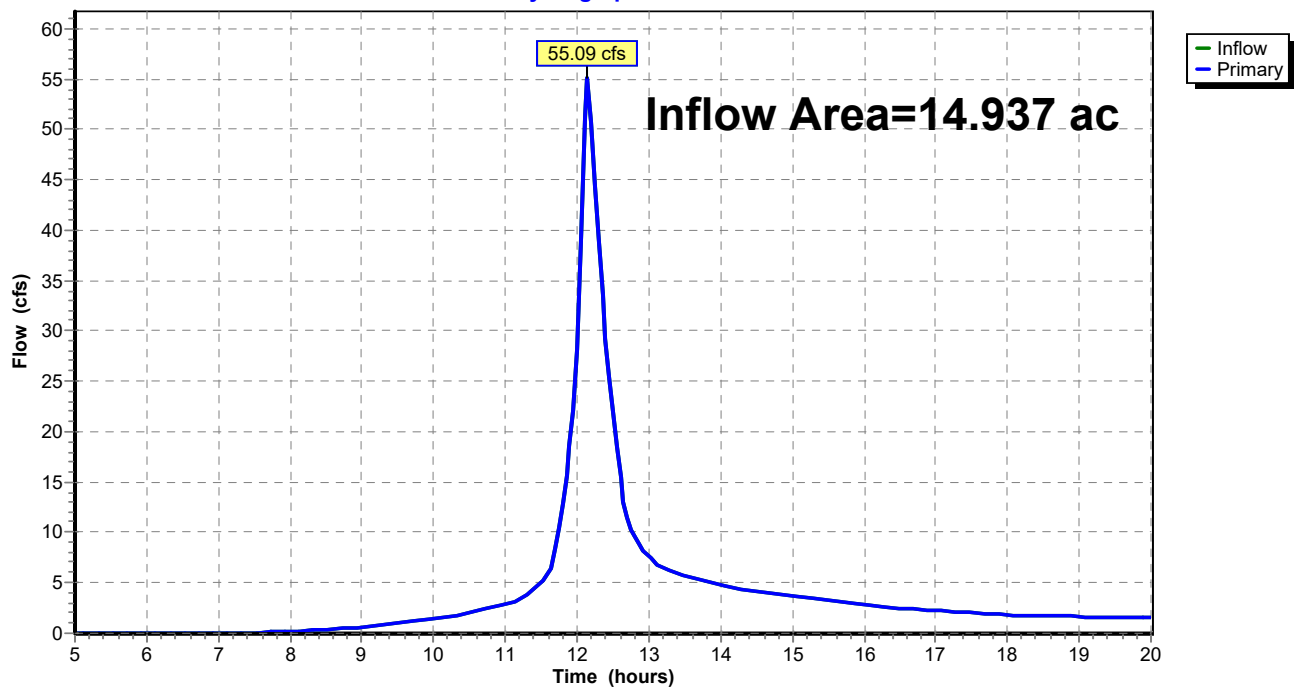
Summary for Link 5L: Post Dev Design Point

Inflow Area = 14.937 ac, 12.18% Impervious, Inflow Depth > 3.87" for 100 YR event
Inflow = 55.09 cfs @ 12.15 hrs, Volume= 4.817 af
Primary = 55.09 cfs @ 12.15 hrs, Volume= 4.817 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link 5L: Post Dev Design Point

Hydrograph



SWM Calcs

Prepared by SK Design Group, Inc

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Stockbridge Subdivision

Type III 24-hr 100 YR Rainfall=7.29"

Printed 12/1/2023

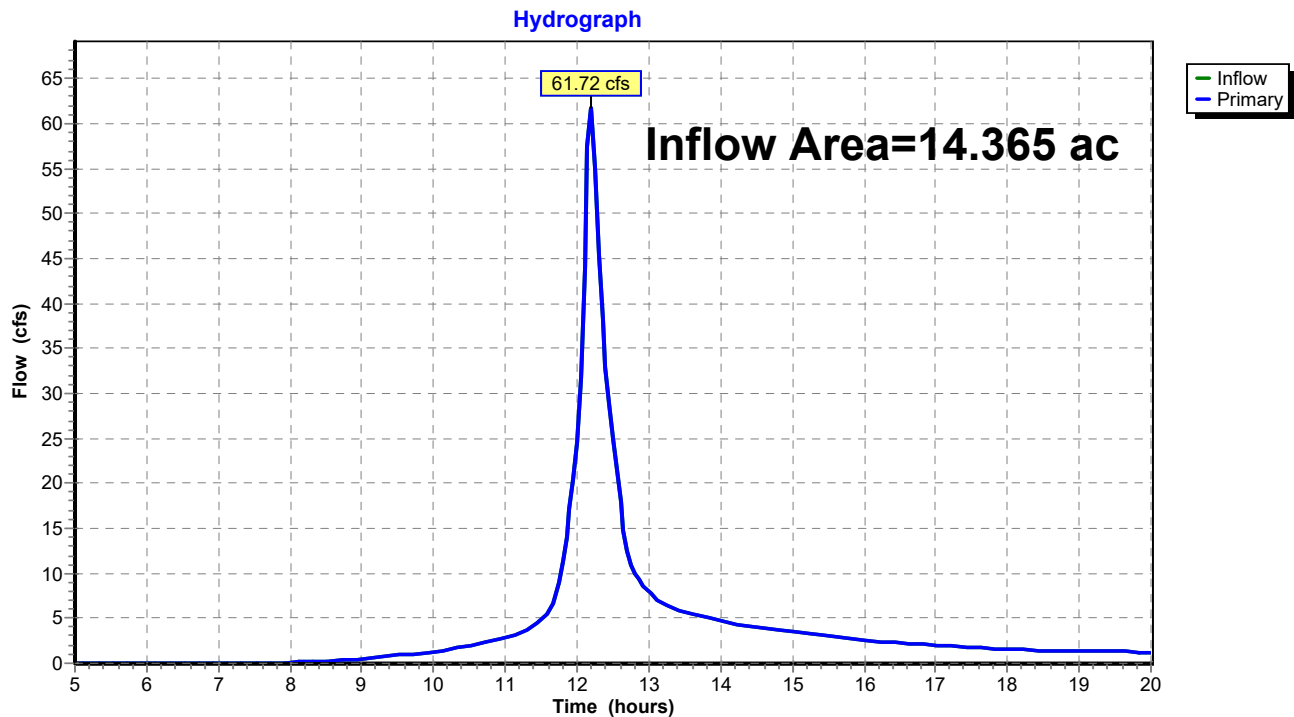
Page 37

Summary for Link DP-E: Design Point Existing

Inflow Area = 14.365 ac, 6.96% Impervious, Inflow Depth > 4.00" for 100 YR event
Inflow = 61.72 cfs @ 12.19 hrs, Volume= 4.788 af
Primary = 61.72 cfs @ 12.19 hrs, Volume= 4.788 af, Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 5.00-20.00 hrs, dt= 0.05 hrs

Link DP-E: Design Point Existing



ATTACHMENT 4

Original Survey Plan
(By Foresight Land Services)

GENERAL NOTES

1. This Plan of Land is intended solely to represent the boundary lines of the subject property.
2. Unless otherwise noted herein, this survey plan shall not be construed as depicting the presence, absence or location of any natural or man-made features, such as streams or ponds, are not represented as building lines of wetland features areas.
3. No other specific, appropriate, same, with conditions or suitability are expressed or implied hereby, either directly or by omission.
4. All parcels are subject to all with the benefit of all rights, easements, servitudes, encumbrances, leases, encumbrances and appurtenances of record.
5. Enforcement does not imply compliance with Wetlands Protection Act or similar.
6. Horizontal Datum is based upon Massachusetts State Plane System, NAD83 Zone 18. Data provided by GPS occupations performed by Clark Engineering & Surveying, P.C.
7. Reference is made to a plan entitled "Definitive Plan, Italian Acres, Stockbridge, Mass., surveyed for Hark, K. Hark, Book 185, Stockbridge, Mass., Sheet 1 of 2, dated 1972, and recorded in the Middlesex County Registry of Deeds (B41050) in Plan Book 41702, Plan 157.
8. Reference is made to a plan entitled "Plan of Land for Friends of Stockbridge School in Stockbridge, Massachusetts, dated March 1972, prepared by Robert C. Brown & Assoc., Inc. Said plan is recorded in the B41050 in Plan Book 41702, Plan 21.
9. Reference is made to a plan entitled "Plan of Land surveyed for Charles H. & Barbara J. Korny in Stockbridge, Massachusetts, June 1989, prepared by Kelly-Grogan-Purton and Associates, Inc. Said plan is recorded in the B41050 in Part C, #29.
10. Reference is made to a plan entitled "Plan of Land in Stockbridge, Massachusetts surveyed for James Mann and Jacqueline Mann, February 7, 1998, prepared by Accord Engineering & Surveying, Inc. Said plan is recorded in the B41050 in Part C, #29.
11. Reference is made to a plan entitled "Definitive Plan of Land in Stockbridge, Mass., prepared for the Mass. Assoc., February 1986, prepared by Brown Associates, Inc. Said plan is recorded in the B41050 in Part C, #29.

Station	Bearing	Distance
1	N 87° 25' 00" E	81.13'
2	N 87° 25' 00" E	81.13'
3	N 87° 25' 00" E	81.13'
4	N 87° 25' 00" E	81.13'
5	N 87° 25' 00" E	81.13'
6	N 87° 25' 00" E	81.13'
7	N 87° 25' 00" E	81.13'
8	N 87° 25' 00" E	81.13'
9	N 87° 25' 00" E	81.13'
10	N 87° 25' 00" E	81.13'
11	N 87° 25' 00" E	81.13'
12	N 87° 25' 00" E	81.13'
13	N 87° 25' 00" E	81.13'
14	N 87° 25' 00" E	81.13'
15	N 87° 25' 00" E	81.13'
16	N 87° 25' 00" E	81.13'
17	N 87° 25' 00" E	81.13'
18	N 87° 25' 00" E	81.13'
19	N 87° 25' 00" E	81.13'
20	N 87° 25' 00" E	81.13'
21	N 87° 25' 00" E	81.13'
22	N 87° 25' 00" E	81.13'
23	N 87° 25' 00" E	81.13'
24	N 87° 25' 00" E	81.13'
25	N 87° 25' 00" E	81.13'
26	N 87° 25' 00" E	81.13'
27	N 87° 25' 00" E	81.13'
28	N 87° 25' 00" E	81.13'
29	N 87° 25' 00" E	81.13'
30	N 87° 25' 00" E	81.13'

VICINITY MAP



I CERTIFY THAT THIS PLAN SHOWS THE BOUNDARY LINES OF THE SUBJECT PROPERTY AS THEY EXIST AT THE TIME OF RECORDING. I HAVE NOT BEEN ADVISED OF ANY CHANGES IN THE BOUNDARY LINES OF THE SUBJECT PROPERTY SINCE THE DATE OF RECORDING. I HAVE NOT BEEN ADVISED OF ANY CHANGES IN THE BOUNDARY LINES OF THE SUBJECT PROPERTY SINCE THE DATE OF RECORDING. I HAVE NOT BEEN ADVISED OF ANY CHANGES IN THE BOUNDARY LINES OF THE SUBJECT PROPERTY SINCE THE DATE OF RECORDING.

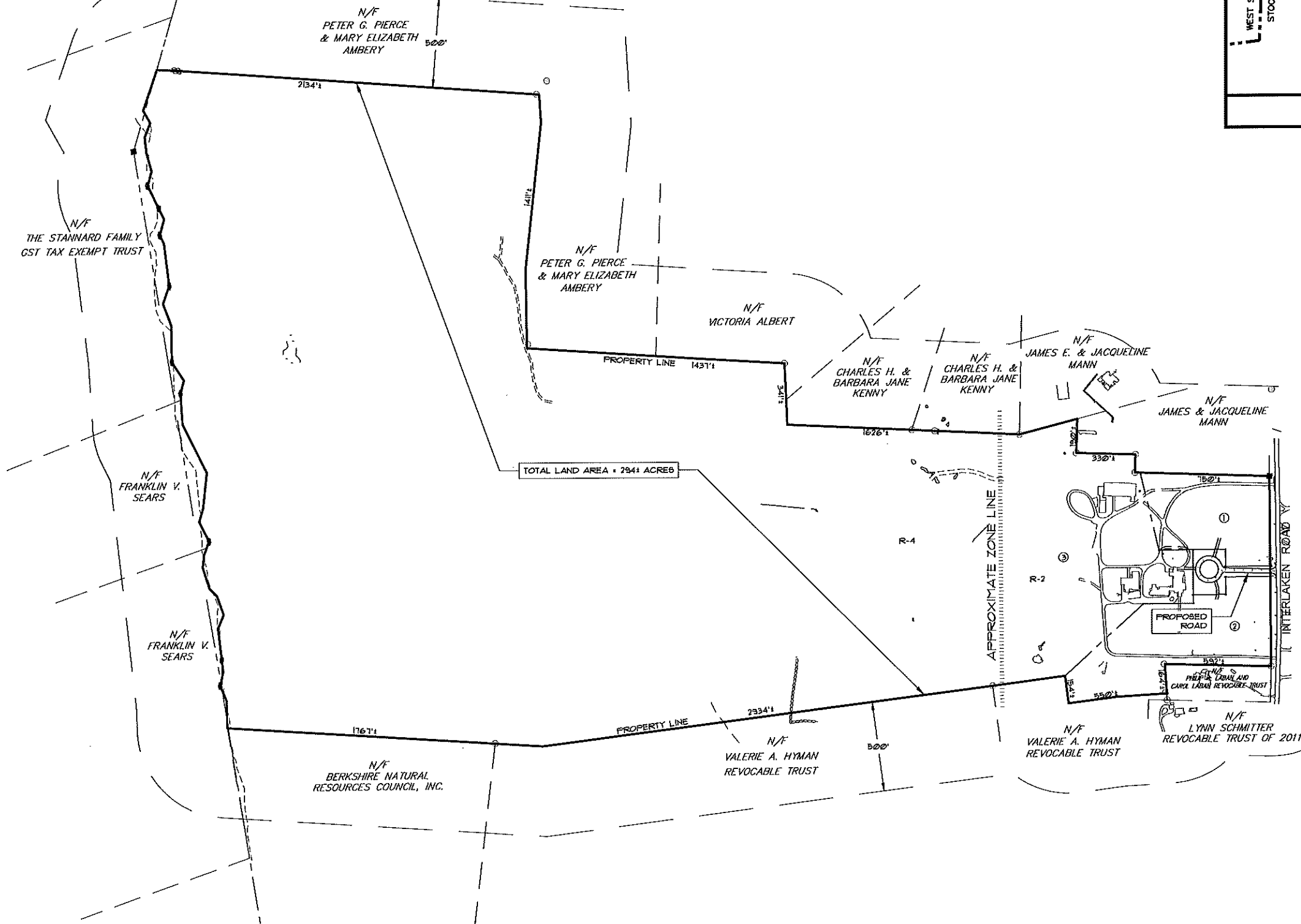
Robert C. Brown
2010-01-14-2012

I CERTIFY THAT THIS PLAN HAS BEEN PREPARED IN ACCORDANCE WITH THE REQUIREMENTS OF MASS. REGS. 801.00:01, 801.00:02, 801.00:03, 801.00:04, 801.00:05, 801.00:06, 801.00:07, 801.00:08, 801.00:09, 801.00:10, 801.00:11, 801.00:12, 801.00:13, 801.00:14, 801.00:15, 801.00:16, 801.00:17, 801.00:18, 801.00:19, 801.00:20, 801.00:21, 801.00:22, 801.00:23, 801.00:24, 801.00:25, 801.00:26, 801.00:27, 801.00:28, 801.00:29, 801.00:30, 801.00:31, 801.00:32, 801.00:33, 801.00:34, 801.00:35, 801.00:36, 801.00:37, 801.00:38, 801.00:39, 801.00:40, 801.00:41, 801.00:42, 801.00:43, 801.00:44, 801.00:45, 801.00:46, 801.00:47, 801.00:48, 801.00:49, 801.00:50, 801.00:51, 801.00:52, 801.00:53, 801.00:54, 801.00:55, 801.00:56, 801.00:57, 801.00:58, 801.00:59, 801.00:60, 801.00:61, 801.00:62, 801.00:63, 801.00:64, 801.00:65, 801.00:66, 801.00:67, 801.00:68, 801.00:69, 801.00:70, 801.00:71, 801.00:72, 801.00:73, 801.00:74, 801.00:75, 801.00:76, 801.00:77, 801.00:78, 801.00:79, 801.00:80, 801.00:81, 801.00:82, 801.00:83, 801.00:84, 801.00:85, 801.00:86, 801.00:87, 801.00:88, 801.00:89, 801.00:90, 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ATTACHMENT 5

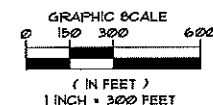
Definitive Subdivision Plans
(Dated Dec. 4, 2023)

D:\SK DESIGN GROUP\2016\160117-Shepton Acquisitions-Former DeSoto School-Site Design & Permitting\Drawings\Subdivision\160117 SUBDIVISION (10-23-2023).dwg
Plot Date: Monday, December 04, 2023 10:35am
User Name: Brandon



1 OVERALL SITE PLAN
SCALE: 1" = 300'

NOTE:
EXISTING CONDITIONS PLAN COMPILED FROM AERIAL TOPOGRAPHY FLYOVER AND PROPERTY LINE
INFORMATION COMPILED FROM PLANS PREPARED BY FORESIGHT LAND SERVICES.

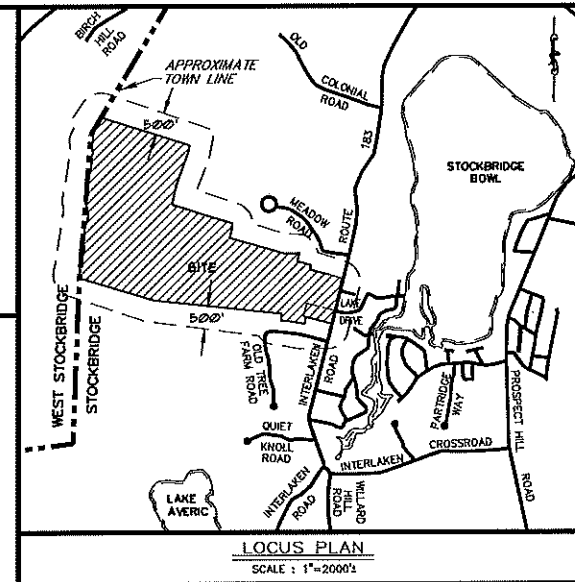


OWNER / APPLICANT:

35-37 INTERLAKEN ROAD REALTY TRUST
PATRICK J. SHEEHAN, TRUSTEE
116 BOYLSTON STREET
UNIT E9A
BOSTON, MA 02119

ENGINEER:

SK DESIGN GROUP, INC.
JAMES M. SCALISE, II
2 FEDERICO DRIVE
PITTSFIELD, MA 01201
413-443-3531



BONNIE BRIER LANE SUBDIVISION

PREPARED FOR:

35-37 INTERLAKEN ROAD REALTY TRUST

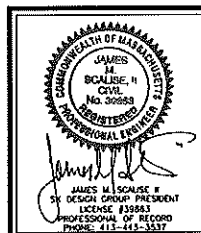
LOCATED AT:

35-37 INTERLAKEN ROAD
STOCKBRIDGE, MASSACHUSETTS



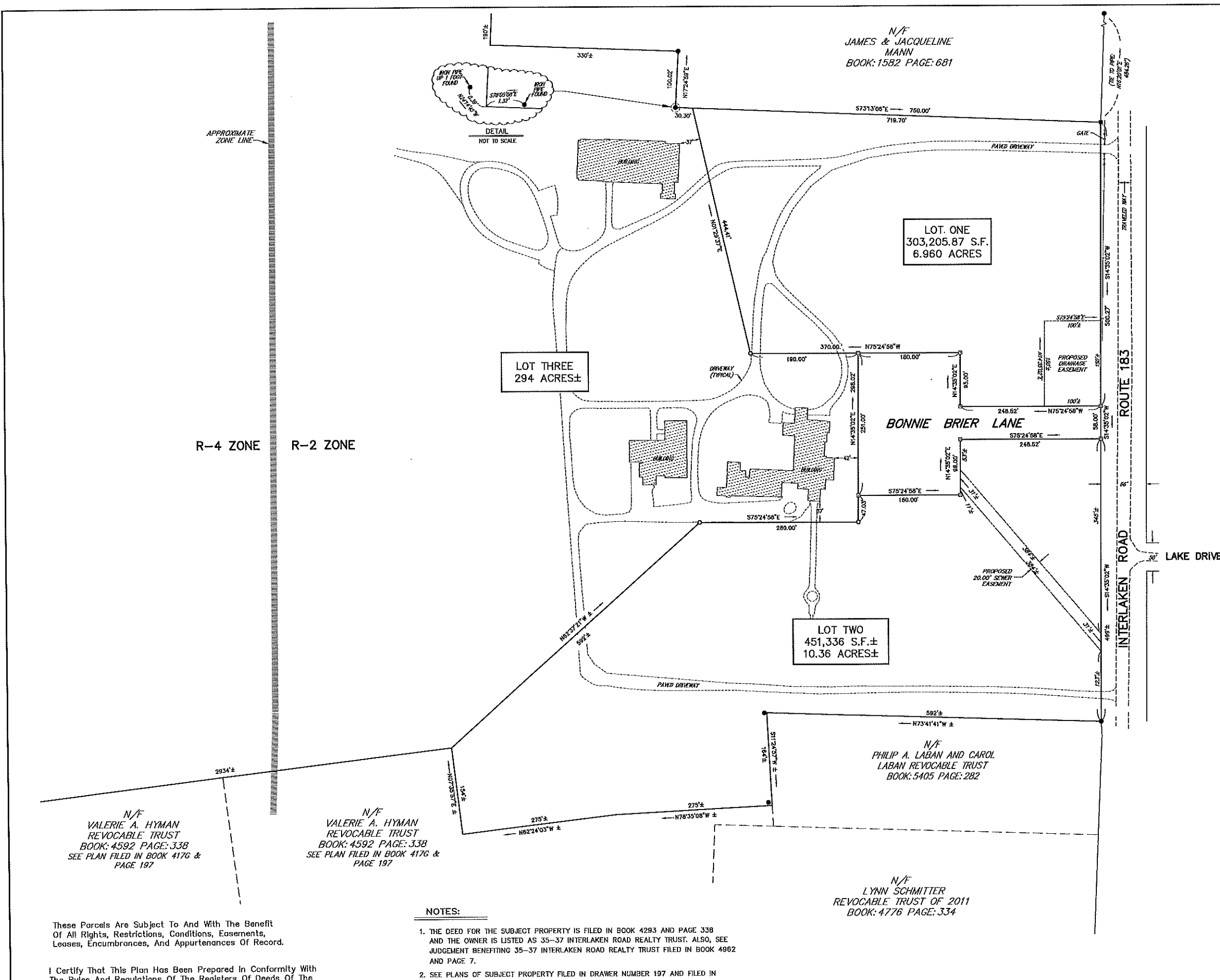
INDEX PLAN

SK DESIGN GROUP PROJECT #:
160117



REVISION:

DESIGNED BY:	CHECKED BY:
BKR	JMS II
DATE:	SHEET NO.
DEC. 4, 2023	1
ISSUED FOR:	OF
PERMIT	6
SCALE:	
AS NOTED	



FILED FOR APPROVAL:
DATE : _____
DATE APPROVED: _____
DATE OF ENDORSEMENT: _____

STOCKBRIDGE PLANNING BOARD

Planning Board Endorsement Does Not Imply Compliance With The MA Wetland Protection Act Or Zoning By-Law.

APPROVED : _____, 20____
HEALTH DEPARTMENT

DATE : _____

I, _____, CLERK OF THE TOWN OF STOCKBRIDGE
HEREBY CERTIFY THAT THE NOTICE OF APPROVAL OF THIS PLAN BY
THE STOCKBRIDGE PLANNING BOARD HAS BEEN RECEIVED AND
RECORDED AT THIS OFFICE AND NO APPEAL WAS RECEIVED
DURING THE TWENTY DAYS NEXT AFTER SUCH RECEIPT AND
RECORDING OF SAID NOTICE.

CLERK

LEGEND

- GRANITE BOUND FOUND
- IRON PIPE FOUND
- BOUND TO BE SET
- IRON PIPE TO BE SET
- POST AND RAIL FENCE

**BUILDING LOT PLAN
OF
BONNIE BRIER LANE**

SURVEYED FOR
**35-37 INTERLAKEN ROAD
REALTY TRUST**

35 AND 37 INTERLAKEN ROAD
STOCKBRIDGE, MASSACHUSETTS

Scale: 0 40 80 Feet
0 10 20 Meters 1"=80'

DECEMBER 4, 2023

Design Group, Inc.
Civil Engineers * Surveyors * Consultants
2 FEDERCO DRIVE * PITTSFIELD, MASSACHUSETTS 01201 * (413)443-3537

DRAWING 2 OF 6 PROJECT No.: 160117-BLP

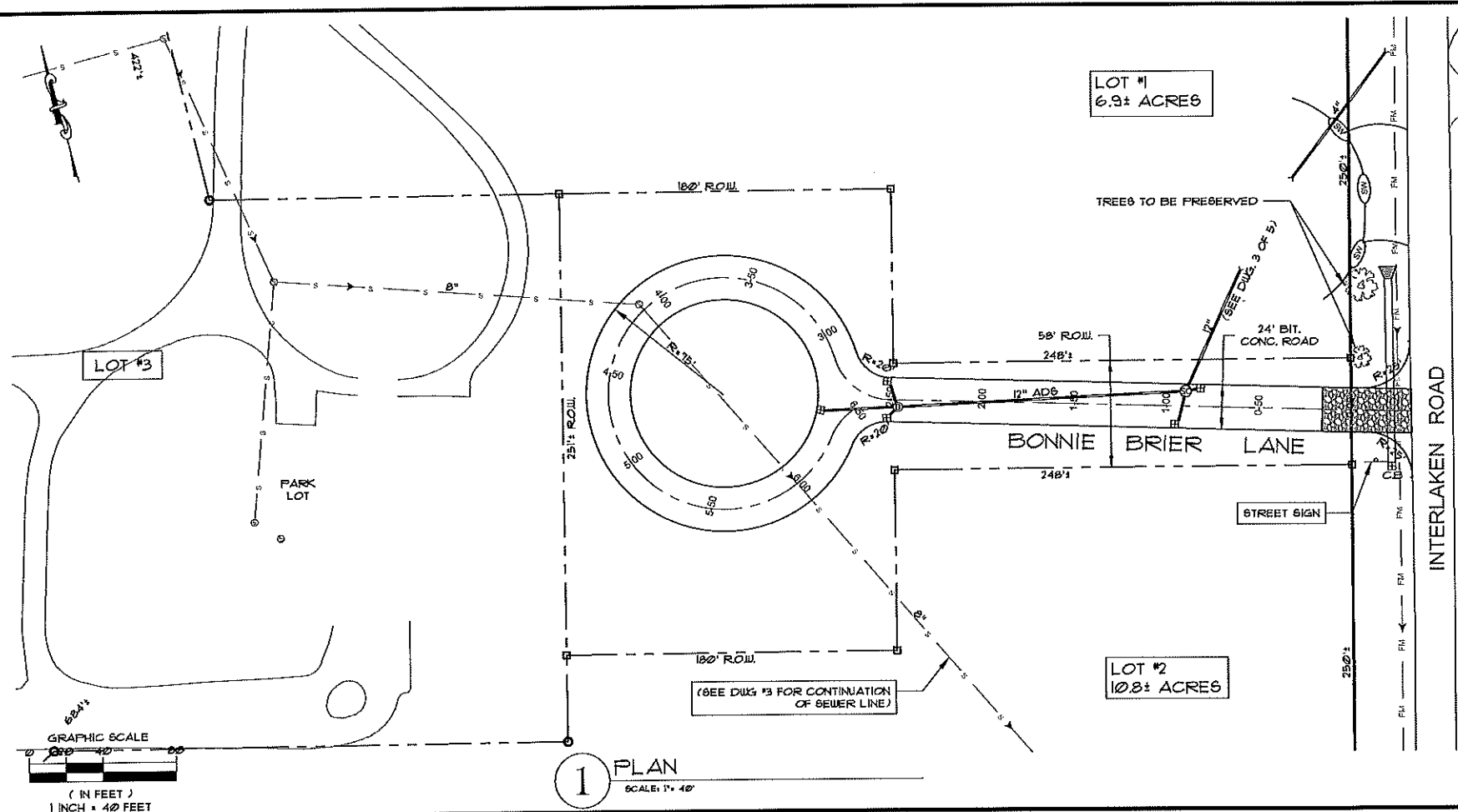
These Parcels Are Subject To And With The Benefit
Of All Rights, Restrictions, Conditions, Easements,
Leases, Encumbrances, And Appurtenances Of Record.

I Certify That This Plan Has Been Prepared In Conformity With
The Rules And Regulations Of The Registers Of Deeds Of The
Commonwealth Of Massachusetts.

Professional Land Surveyor

- NOTES:**
1. THE DEED FOR THE SUBJECT PROPERTY IS FILED IN BOOK 4293 AND PAGE 338
AND THE OWNER IS LISTED AS 35-37 INTERLAKEN ROAD REALTY TRUST. ALSO, SEE
JUDGEMENT BENEFITTING 35-37 INTERLAKEN ROAD REALTY TRUST FILED IN BOOK 4962
AND PAGE 7.
 2. SEE PLANS OF SUBJECT PROPERTY FILED IN DRAWER NUMBER 197 AND FILED IN
PLAT K AND PLAN NUMBER 65.
 3. THE SUBJECT PROPERTY FALLS WITHIN THE R2 AND R4 ZONES AT THE TIME THIS
PLAN WAS PREPARED. LOTS ONE AND TWO FALL ENTIRELY WITHIN THE R-2 ZONE.

SK DESIGN GROUP PROJECT #1: 160117 SUBDIVISION (10-23-2023).dwg
DESIGNER: JESSICA CHAPMAN, 160117-Subdivision-Site Design & Permitting Drawings-Subdivision
CHECKED BY: JAMES M. SCALISE, II, 01/11/2023
DATE: Monday, December 04, 2023 - 9:53am
PROJECT: 160117-Subdivision-Site Design & Permitting Drawings-Subdivision
DRAWN BY: JESSICA CHAPMAN
DATE: Monday, December 04, 2023 - 9:53am
PROJECT: 160117-Subdivision-Site Design & Permitting Drawings-Subdivision



- LEGEND**
- APPROXIMATE PROPERTY LINE
 - EXISTING CONTOUR
 - EXISTING TREELINE
 - EXISTING UTILITY POLE
 - PROPOSED DRAIN MANHOLE
 - PROPOSED CATCH BASIN
 - PROPOSED DRAIN LINE (SIZE AS NOTED)
 - PROPOSED CONCRETE BOUND
 - EXISTING SEWER MAIN
 - EXISTING FORCEMAIN
 - PROPOSED STORMCEPTOR

FILED FOR APPROVAL _____, 2021

APPROVED _____, 2021

STOCKBRIDGE PLANNING BOARD

APPROVED _____, 2021

CONSERVATION COMMISSION

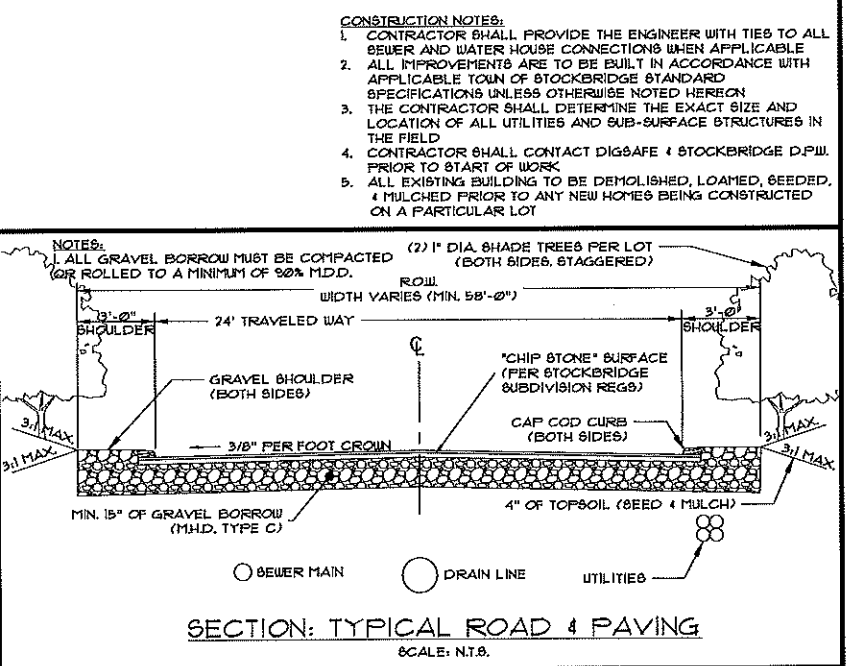
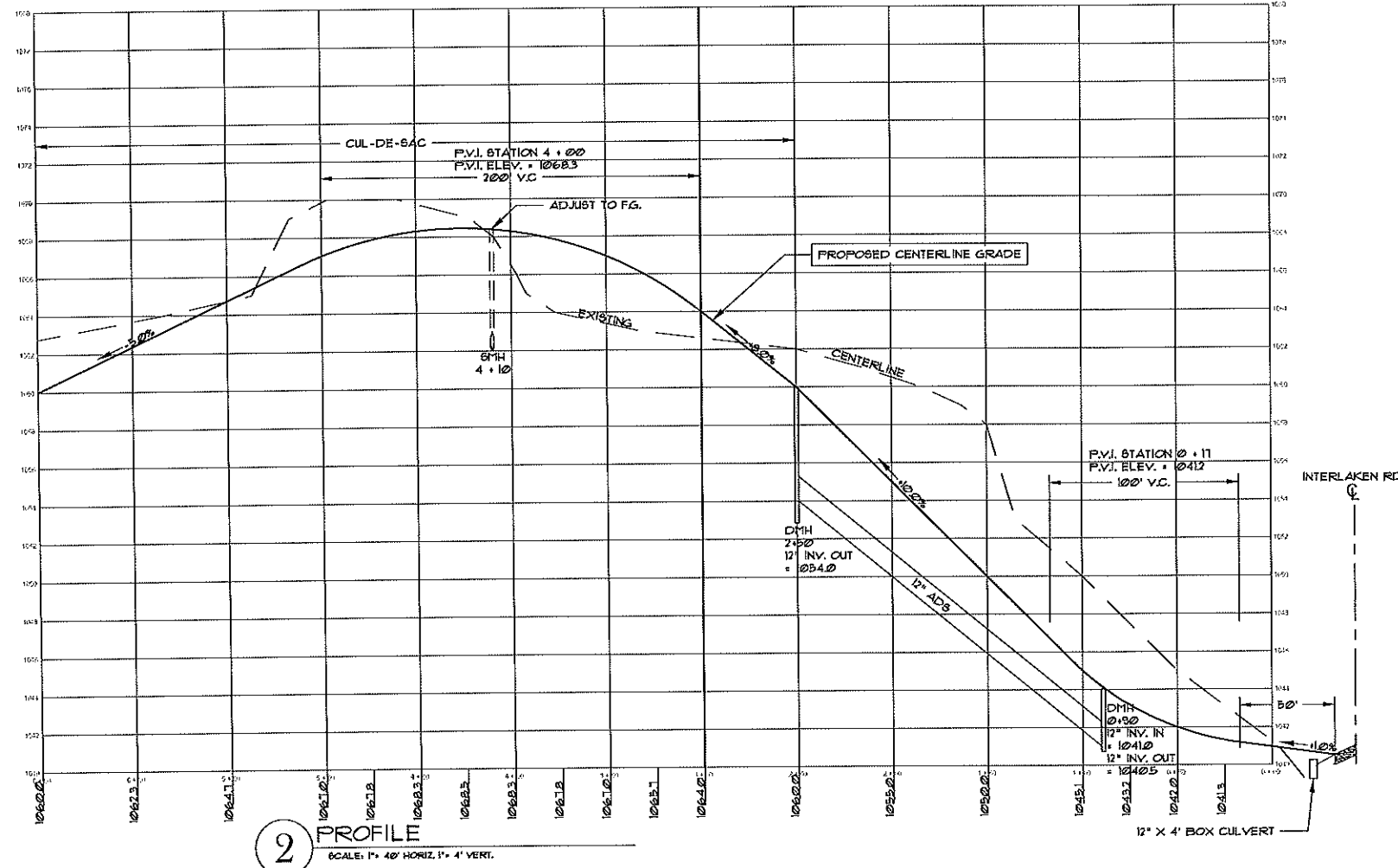
APPROVED _____, 2021

BOARD OF HEALTH

APPROVED _____, 2021

TOWN CLERK

BONNIE BRIER LANE SUBDIVISION
PREPARED FOR:
35-37 INTERLAKEN ROAD REALTY TRUST
LOCATED AT:
35-37 INTERLAKEN ROAD
STOCKBRIDGE, MASSACHUSETTS



Design Group, Inc.
Civil Engineers & Surveyors & Consultants
2 FERRIS DRIVE • PITTSFIELD, MASSACHUSETTS 01201 • TEL: 405-2577

PLAN & PROFILE

SK DESIGN GROUP PROJECT #1: 160117

REVISIONS:

NO.	DATE	DESCRIPTION
1	DEC 4, 2023	ISSUED FOR PERMIT

DRAWN BY: JKR
CHECKED BY: JMS II
DATE: DEC 4, 2023
ISSUED FOR: PERMIT
SCALE: AS NOTED

SHEET NO. 3 OF 6



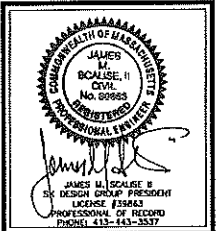
_____ 200' _____ EXISTING CONTOUR
 _____ S _____ S EXISTING SEWER LINE (SIZE AS NOTED)
 _____ FM _____ FM EXISTING SEWER FORCE MAIN
 _____ (SIZE AS NOTED)
 _____ S _____ S PROPOSED SEWER SERVICE (4")
 (P) PROPOSED SEWER PUMP
 _____ EXISTING PROPERTY LINE
 _____ PROPOSED PROPERTY LINE

BONNIE BRIER LANE SUBDIVISION
PREPARED FOR:
35-37 INTERLAKEN ROAD REALTY
LOCATED AT:
35-37 INTERLAKEN ROAD
STOCKBRIDGE, MASSACHUSETTS

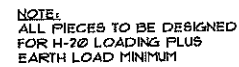
Design Group, Inc.
Civil Engineers * Surveyors * Consultants
2 FIDELITY DRIVE * WITTSFIELD, MASSACHUSETTS 01201 * (413) 442-3557

PLAN DESCRIPTION	WATERSHED PLAN
------------------	----------------

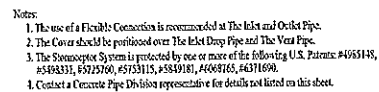
SK DESIGN GROUP PROJECT #: 160117



REVISION: 	
DRAWN BY: BKR	CHECKED BY: JWS II
ORD. DATE: DEC. 4, 2023	SHEET NO. 5 OF 6
ISSUED FOR: PERMIT	
SCALE: 1" = 50'	



3 DETAIL:
TYPICAL UTILITY TRENCH
SCALE: N.T.S.



5 STC 4501 - STORMCEPTOR DETAIL
8GALE: NTB

BONNIE BRIER LANE SUBDIVISION

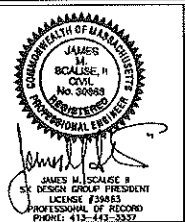
35-37 INTERLAKEN ROAD REALTY TRUST

35-31 INTERLAKEN ROAD
STOCKBRIDGE, MASSACHUSETTS

Design Group, Inc.
Civil Engineers * Surveyors * Consultants
2 FEDERICO DRIVE * PITTSFIELD, MASSACHUSETTS 01201 * (617) 421-1100

DETAILS

SK DESIGN GROUP PROJECT 1:
16017



Stone:

DRAWN BY: BKR	CHECKED BY: JMS II
DESIGNED DATE: DEC. 4, 2023	SHEET NO. 6
DESIGNED FOR: PERMIT	OF
SCALE: AS NOTED	6

\\FSK DESIGN GROUP\2016\160117-Sheehan Acquisitions-Former DeStigo School-Site Design & Permitting\Drawings\Subdivision\160117 SUBDIVISION (10-23-2023).dwg
 Plotted On: Monday, December 04, 2023 - 9:54am
 User: Narm: B'Rondeau